

Craftsman Village of the Crossroads At Mountain View Meadows Phases 8-10 Traffic Impact Study Update

Helena, Montana



Prepared For:

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Craftsman Village of the Crossroads At Mountain View Meadows Phases 8-10 Traffic Impact Study UPDATE Helena, Montana

A. EXECUTIVE SUMMARY

The Craftsman Village of the Crossroads at Mountain View Meadows Phases 8-10 is a 40-acre residential development located south of US Highway 12/287 in Helena, Montana. The project would consist of 230 single family residential units and would be accessed from Alpine View Drive and Jeannette Rankin Drive. As proposed Craftsman Village of the Crossroads would produce 2,169 new daily trips in the area at full build-out. Craftsman Village of the Crossroads Phases 8-10 will not affect roadway operations in the area. All nearby intersections will continue to function at acceptable levels of service with the proposed development. No roadway modifications are currently recommended with this project.

B. PROJECT DESCRIPTION

This document studies the possible effects on the surrounding road system from a proposed 40-acre residential development located south of US Highway 12/287 within the City of Helena between Helena and East Helena. The document identifies any traffic mitigation efforts that the development may require. The site is located north of Jeannette Rankin Drive and east of Alice Street.

C. EXISTING CONDITIONS

The proposed development property currently consists of a 40-acre parcel of undeveloped land located south of US Highway 12/287 and north of Jeannette Rankin Drive adjacent to Mountain View Park and the previous phases of the Craftsman Village of the Crossroads which are nearing completion. The topography in this area consists of rolling hills which slope downward to the east. See **Figure 1** for a location map of the proposed development.

Adjacent Roadways

US Highway 12/287 is a four-lane, two-way east/west highway which extends east from Helena. US Highway 12/287 has a five-lane cross-section and the posted speed limit near Crossroads Parkway is 55 MPH which decreases to 45 MPH approximately 0.5 miles west of Crossroads Parkway and at the East. The Highway intersects with Crossroads Parkway at a signalized intersection. Crossroads Parkway has a separated eastbound lane from

Highway 287 at the traffic signal. Traffic counts collected in 2021 by Montana Department of Transportation (MDT) indicate that this section of roadway carries an Average Daily Traffic (ADT) volume of 17,200 vehicles per day (VPD) west of Crossroads Parkway.

Highway 282 is a two-way north/south highway which extends south from Highway 12/287 in East Helena to an overpass at I-15. Highway 282 has a two-lane rural cross-section with a paved width of 28 feet. Near Runkle Parkway the highway has an extended width accommodating a northbound left-turn lane at the intersection. The posted speed limit near Runkle Parkway is 45 MPH and increases to 60 MPH south of Runkle Parkway. Traffic counts collected in 2021 by MDT indicate that this section of roadway carries an Average Daily Traffic (ADT) volume of 1,900 vehicles per day (VPD) south of Manlove Street.

Crossroads Parkway is a north/south, four-lane roadway that extends south from US Highway 12/287 providing residential and commercial access in the area. The road has an urban cross-section with a paved width of 65 feet, which includes left-turn bays at each intersection. The roadway features a central median and has a posted speed limit of 35 MPH. Crossroads Parkway is signal controlled at the intersection with US Highway 12/287.

Alice Street is a two-lane road which provides access to developments south of US Highway 12/287. Alice Street starts in an east/west direction at its intersection with Crossroads Parkway and changes to north/south approximately 0.25 miles east of Crossroads Parkway. The street has variable widths (24, 30, 36 and 40 feet) due to bulb outs along its north/south direction. The posted speed limit on Alice Street is 35 MPH. Data collected by Abelin Transportation Services (ATS) in 2022 indicates that the roadway currently carries 2,100 VPD.

Jeannette Rankin Drive is an east/west, two-lane residential collector street which intersects Alice Street to provide access to homes and the Mountain View Park. Jeannette Rankin Drive has a paved width of 33 feet and is a thru-street with STOP signs at each intersection. Jeannette Rankin Drive has a STOP sign at its intersection with Alice Street. Data collected by ATS in 2022 indicates that the roadway currently carries say 500 VPD.

Runkle Parkway is an east/west, two-lane roadway that extends west from Highway 282 providing residential access in the area. The road has an urban cross-section with a paved width of 42 feet and a posted speed limit of 35 MPH. Runkle Parkway has a central median with designated left-turn lanes at all cross-streets. Runkle Parkway is STOP controlled at the tee-intersection with Highway 282.

Alpine View Drive is a north/south, two-lane road which intersects Runkle Parkway to provide access to the developments in the area. The roadway is currently under construction to City of Helena local road standards.



Figure 1- Proposed Development Site

Traffic Counts

In July 2022 ATS collected traffic data to evaluate current operational characteristics. The data collected includes a peak-hour turning movement count performed at the intersection of Jeannette Rankin Drive and Alice Street and 48-hour traffic counts along Alice Street and Jeannette Ranking Drive. Additional data used for this project was obtained from a February 2021 traffic counting effort by ATS at the intersections of Highway 282 with Runkle Parkway and Alpine View Drive. Based on comments from the City of Helena, ATS provided an additional review of the intersection of US 12/287 and Crossroads Parkway from November 2022. The raw traffic data is included in **Appendix A** of this report.

Raw traffic data is typically adjusted for seasonal variation in accordance with the data collected from MDT's permanent traffic data located on Custer Avenue east of York Road (Station A-079). This count station data indicated that data collected in July 2022 is approximately 116% of the Average Annual Daily Traffic (AADT) in this area. For a conservative result no factorization applied was to the July traffic data. Based on the information from Station A-079, the data collected in November 2022 should be approximately 92% of the AADT. The raw data collected in November was factored accordingly. It should also be noted that the existing traffic volumes represent a significant amount of service traffic related to aggressive homebuilding.

ATS obtained historic traffic data for US Highway 12 and MT Highway 282 from the MDT. This data is presented in **Table 1**. Based on the available traffic data for these two roadways, traffic volumes in this area have not increased significantly over the past ten years.

Table 1 - Historic Average Daily Traffic Data (Source: MDT)

Location	2012	2013	2014	2015	2016	2017	2018	2019	2020	2021
Hwy 282 S of										
Manlove St										
#25-7B-044					1,596	1,583	1,917	1,929	1794	1795
Hwy 12/287 W										
of Wylie Dr										
#25-7B-019	18,660	17,520	17,920	17,000	16,540	17,099	17,338	17,251	15,733	17,196

Vehicle speed and volume data was also collected along Alice Street during the July count period. This information suggested that the average vehicle speed on Alice Street north of Jeannette Rankin Drive was 30 MPH with an 85th percentile speed of 37 MPH for all recorded vehicles. In general, vehicle travel speeds on this section are near or slightly above the posted 35 MPH speed limit.

Area Crash Data

ATS obtained crash data from the MDT vehicle crash database for the study intersections. The data included all reported crashes which occur on these segments of road over the past five years. The MDT database indicates that 16 crashes have occurred at the intersection of HWY 287 and Crossroads Parkway and two crashes have occurred at the intersection of Runkle Parkway and MT 282. No crashes were reported at the other study intersections over the five-year period. Crash rates for HWY 287/Crossroads Parkway and Runkle Parkway/MT 282 are 0.03 per MVM and 0.4 respectively. These rates are well within typical standards and do not indicate a need for roadway mitigation.

Additional Projects

Aspen View Condominiums Phase 1 & 2 are currently under construction just north of Runkle Parkway along Alpine View Drive. The projects will include 58 condominium units and would produce 424 VPD from both phases. The projected future traffic from this project is included with the overall analysis for the Craftsman Village development.

The Craftsman Village at Mountain View Meadows generally matches the original intent of the residential density in this portion of the development property. Currently Mountain View Meadows has developed 400 of the originally planned 1,055 residential units on the property. The developers plan to amend a portion of the original 2012 Crossroads Preliminary Plat to update and correct the development plans and account for current lot configurations which includes reduction in overall lot density. There are no other MVM planned projects at this time that have not been previously analyzed with the existing preliminary plat regulatory review. For analysis purposes, a 5% growth rate was applied to the existing traffic volumes to account for the estimated growth for other approved projects at the project site through 2025. The approved projects include the Crossroads Amendment at Mountain View Meadows as approved by the City in 2012.

Overall, this analysis conservatively accounts for an additional 20 housing units per year in addition to the complete build out of the Aspen View Condominiums Phase 1 & 2 and the full buildout of the Craftsman Village of the Crossroads at Mountain View Meadows Phases 8-10. It should be noted, as previously described, the existing traffic volumes represent a significant amount of home building service traffic, thus a direct correlation to occupied housing units is speculative.

Level of Service

Using the data collected for this project, ATS conducted a Level of Service (LOS) analysis at area intersections. This evaluation was conducted in accordance with the procedures outlined in the Transportation Research Board's *Highway Capacity Manual 7th Edition: A Guide for Multimodal Mobility Analysis* and the Highway Capacity Software (HCS) version 8.2. Intersections are graded from A to F representing the average delay that a vehicle entering an intersection can expect. Typically, a LOS of C or better is considered acceptable for peak-hour conditions.

Table 2 shows the existing 2022 LOS for the AM and PM peak hours without the traffic from the proposed Craftsman Village of the Crossroads Phases 8-10. The LOS calculations are included in **Appendix C**. The table shows that the existing intersections along Alice Street and Runkle Parkway are currently operating within acceptable limits. No intersection modifications are currently needed in this area to improve capacity. ATS also developed a no-build scenario for the study roadways based on a 1.7% traffic growth rate

over the next three years. The projected no-build LOS is nearly identical to the existing traffic conditions. The results of this analysis are shown in **Table 3**.

Table 2 – 2022 Level of Service Summar	ry (Source: ATS)
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	AM Pea	ak Hour	PM Peak Hour				
Intersection	Delay (Sec.)	LOS	Delay (Sec.)	LOS			
Highway 287 & Crossroad Pkwy	8.7	Α	5.5	Α			
Highway 282 & Runkle Pkwy*	10.8	В	9.0	Α			
Runkle Parkway & Alpine View*	9.0	Α	8.7	Α			
Alice Street & Jeannette Rankin*	9.2	Α	9.0	Α			

^{*}Northbound/Southbound LOS and Delay or Eastbound/Westbound Side Street LOS and Delay.

Table 3 – 2025 No-Build Level of Service Summary (Source: ATS)

	AM Pea	k Hour	PM Peak Hour					
Intersection	Delay (Sec.)	LOS	Delay (Sec.)	LOS				
Highway 287 & Crossroad Pkwy	9.3	Α	5.7	Α				
Highway 282 & Runkle Pkwy*	11.0	В	9.1	Α				
Runkle Parkway & Alpine View*	9.0/9.5	A/A	8.8/8.7	A/A				
Alice Street & Jeannette Rankin*	9.2	Α	9.1	Α				

^{*}Northbound/Southbound LOS and Delay or Eastbound/Westbound Side Street LOS and Delay.

The City of Helena also requested reviews of the possible traffic impacts from the proposed project to the intersection of US 12/287 with Elaine Street, Lola Street, North Side Frontage Road Access, Nickle Street, and South Side Frontage Road Access. The Helena LRTP indicates that several of these intersections have existing LOS deficiencies that should be addressed in the future. TSM-21 in the LRTP recommends installation of future traffic signals on US Highway 12 at Lola Street and/or Nicole Street when signal warrants are met. These roadway improvements have not been completed. The development of the Craftsman Village at Mountain View Meadows does not directly access any of these roadways and will not directly impact the operations of these intersections other than contributing to the ongoing traffic volume growth along the US 12/287 corridor. The intersections are also not part of the City of Helena major street network and are well over a mile from the proposed development. The traffic impacts from the Craftsman Village at these intersections will be minimal. The LRTP does not contain any additional recommended roadway improvements that would apply to the study area.

D. PROPOSED DEVELOPMENT

The development currently under consideration for this site includes 40 acres of land located west of the previous phases of the Craftsman Village of the Crossroads development and bordered by Jeannette Rankin Drive and Alpine View Drive. Phases 8-10 of the development would include up to 230 single-family residential properties. The project is planned for the development of 70 lots in phases 8 and 9, and 90 lots in Phase 10. Each phase would take one year to complete from 2023 to 2025. Access to the site would be provided through existing approaches from Jeannette Rankin Drive to the west and Alpine View Drive and a new connection to Alice Street at Alpine View Drive would be constructed with Phase 10. These phases of the project are expected to reach full development by 2025. All roads within the development would be constructed to City the Helena standards and will include boulevard sidewalks. On- and off-street parking will be supplied per City of Design Standards. All new streets will include sidewalks or a bike path according to the City complete streets policy and a 10-foot paved path adjacent to Runkle Parkway and Alice Street is being developed along Alpine View Drive. This path will continue west from Alice Street along Runkle Parkway. There are currently no public transit stops within the vicinity of Mountain View Meadows. The Craftsman Village of the Crossroads phases 8-10 is shown in Figure 2.

E. TRIP GENERATION AND ASSIGNMENT

ATS performed a trip generation analysis to determine the anticipated future traffic volumes from the proposed development using the trip generation rates contained in *Trip Generation* (Institute of Transportation Engineers, Eleventh Edition). These rates are the national standard and are based on the most current information available to planners. A vehicle "trip" is defined as any trip that either begins or ends at the development site. ATS determined that the critical traffic impacts on the intersections and roadways would occur during the weekday morning and evening peak hours. According to the ITE trip generation rates, the Craftsman Village of the Crossroads would produce 161 AM peak hour trips, 216 PM peak hour trips, and 2,169 daily trips. See **Table 4** for detailed trip generation information.

Table 4 - Trip Generation Rates (Source: ATS)

Single-Family Res. ITE #210	Units	AM Peak Hour Trip Ends per Unit	Total AM Peak Hour Trip Ends	PM Peak Hour Trip Ends per Unit	Total PM Peak Hour Trip Ends	Weekday Trip Ends per Unit	Total Weekday Trip Ends
Phase 8 – 2023	70	0.7	49 (13in/36out)	0.94	66 (42in/24out)	9.43	660
Phase 9 – 2024	70	0.7	49 (13in/36out)	0.94	66 (42in/24out)	9.43	660
Phase 10 - 2025	90	0.7	63 (16in/47out)	0.94	85 (54in/31out)	9.43	849
TOTAL	230		161 (42in/119out)		217 (138in/79out)		2,169

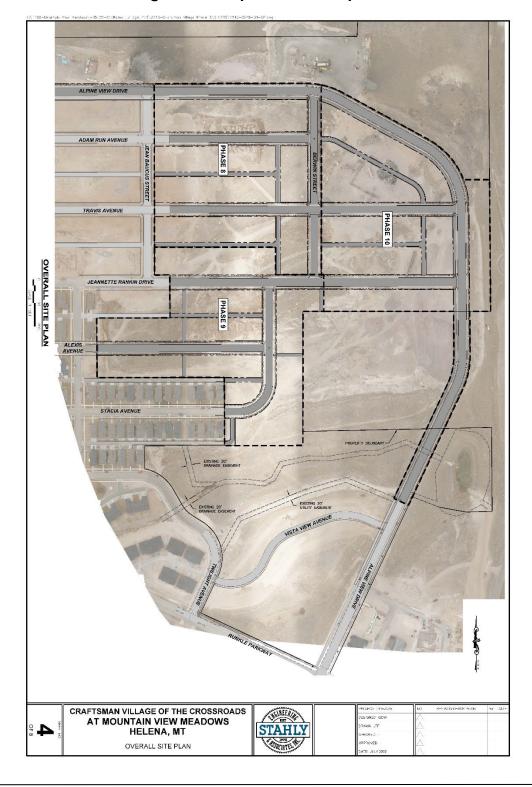


Figure 2 – Proposed Development

F. TRIP DISTRIBUTION

The traffic distribution and assignment for the proposed subdivision was based upon the existing ADT volumes along the adjacent roadways and the peak-hour turning volumes. It is expected that 75% of the traffic from the proposed development site would use Jeannette Rankin Drive and Alpine View Drive to reach Alice Street and Highway 287 to the west into Helena, 15% would distribute to the south onto Runkle Parkway and Highway 282 to the south, and 10% would distribute north onto Highway 282 towards East Helena. All traffic directing towards Helena from Phases 8 and 9 of the development would use Jeanette Rankin Drive. This traffic will decrease significantly in Phase 10 with the direction connection of Alpine View Drive to Alice Street. Traffic is expected to distribute onto the surrounding road network as shown on **Figure 3**.

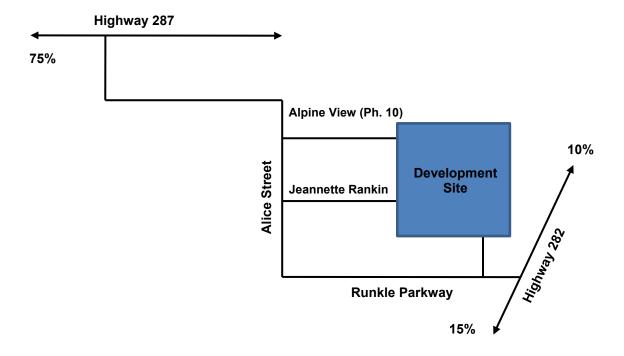


Figure 3 – Trip Distribution

G. TRAFFIC IMPACTS OUTSIDE OF THE DEVELOPMENT

Using the trip generation and trip distribution numbers, ATS determined the future Level of Service for the area intersections. The anticipated intersection LOS with the Craftsman Village of the Crossroads Phases 8-10 is shown in **Tables 5**. These calculations are based on the projected model volumes included in **Appendix B** of this report and includes the projected traffic from the 58 units from the Aspen View Condominiums Phases 1&2 currently under construction.

Table 5 indicates that the construction of the Craftsman Village of the Crossroads Phases 8-10 will have little effect on the traffic conditions within the area. All area intersections will continue to function at LOS B or better at full build-out and no additional mitigation measures will be needed to improve intersection capacity. Both the intersections of Twilight Avenue and Alpine View Drive with Runkle Parkway have existing center left-turn lanes which will provide reserve operational capacity for these intersections well into the future. Based on the existing and projected traffic volumes along Alice Street, no additional turning lanes would be required at the intersections with Alpine View Drive or Jeannette Rankin Drive.

Total traffic volumes on Alice Street and Crossroads Parkway will increase by 1,600 VPD to a total of 3,700 VPD which is well within the capacity of a collector roadway. Traffic volume along the southern portion of Alpine View Drive will increase by approximately 500 VPD. The project will initially increase traffic volumes along Jeannette Rankin Drive (collector roadway) by approximately 1,000 VPD with Phases 8 & 9 which will increase the total traffic along Jeannette Rankin Drive to 1,500 VPD. With the connection of Alpine View Drive to Alice Street with Phase 10 of the project, traffic volumes along Jeannette Rankin Drive will decrease to approximately 800 VPD. It should be noted that Crossroads Parkway is currently constructed with an urban three-lane cross-section which has the capacity to handle 18,000 VPD to 20,000 VPD. The road is currently carrying 2,000 VPD to 3,000 VPD, or 10-20% of the roadway's capacity.

Table 5 –Level of Service Summary
With the Craftsman Village of the Crossroads Phases 8-10 (Source: ATS)

	AM Pea	ak Hour	PM Pea	ak Hour
Intersection	Delay (Sec.)	LOS	Delay (Sec.)	LOS
Highway 287 & Crossroad Parkway	12.7	В	11.5	В
Highway 282 & Runkle Parkway	11.4	В	9.7	А
Runkle Parkway & Alpine View*	9.1/9.8	A/A	8.9/8.9	A/A
Alice Street & Alpine View	9.6	А	9.5	А
Alice Street & Jeannette Rankin	9.4	А	9.3	А

^{*}Northbound/Southbound LOS and Delay.

ATS used the HCS data to identify the average and maximum vehicle queuing lengths that can be expected at the study intersections with the development of the Craftsman Village. Under the existing roadway configuration, northbound vehicles queue at the Highway 287/Crossroads Parkway intersection will be in the range of 200 to 300 feet which are below the provided turn lane length (320 feet). The other turn lanes including the intersection of MT 282 with Runkle Parkway will have an average vehicle queue length of less than one vehicle.

Reviewing the traffic forecasts and relation to intersection LOS analysis, the intersections of Highway 12 and Crossroads Parkway, and the intersection of Highway 282 and Runkle Parkway can accommodate an excess amount of traffic from the total planned MVM projects, estimated at 1,055 housing units, while continuing to function within an acceptable LOS. ATS ran a LOS analysis for the intersections of Highway 12 and Crossroads Parkway and the intersection of Highway 282 and Runkle Parkway DOUBLING the currently anticipated traffic volumes at these intersections from the MVM. The analysis showed that both intersections would still function at LOS C or better with twice as much traffic as is currently projected at full buildout in 2025. The LOS analysis for this condition is included with this report. As MDT continually monitors the traffic signal operations at Highway 12 and Crossroads Parkway, signal timing adjustments for traffic flow optimization may be needed at before full buildout of the properties in this area to accommodate the continued growth on the subject properties, but no geometric improvements would be necessary to support full buildout of the planned projects.

H. IMPACT SUMMARY & RECOMMENDATIONS

As proposed Craftsman Village of the Crossroads would produce 2,169 new daily trips in the area at full build-out. Craftsman Village of the Crossroads Phases 8-10 will not affect roadway operations in the area. All nearby intersections will continue to function at acceptable levels of service with the proposed development. No roadway modifications are currently recommended with this project.

APPENDIX A

Traffic Data

Turning Movement Count All Vehicles

Location MountainView
Date 7-19 PM and 7-20 AM

Date	7-13 1 W	and i		bound			Southb	ound			Eastbo	ound			Westbo	ound		
		Left	Thr		Peds	l eft			Peds	l eft	Thr		Peds	Left			Peds	ΤΩΤΔΙ
7:00 -	7:15	(0	0	0	0	0		0	0			0		0
7:15 -						5	4	0	-	0	-	0	0	_	-	7	-	29
7:30 -		(5	6	0	-	0		0	0			6	-	36
7:45 -						9	7	0	-	0		0	0	2		8		50
8:00 -						5	6	0	0	0	-	0	0	1	0	3		30
8:15 -						9	3	0	-	0	-	0	0	Ö		8		34
8:30 -						9	11	0	-	0	-	0	0	0	-	3		39
8:45 -						0	0	0	-	0	-	0	0	0	-	0		0
9:00 -						0	0	0	-	0		0	0			0	-	0
9:00 - 9:15 -						0	0	0	-	0		0	0			0	-	0
9:30 -						0	0	0	0	0		0	0			0		0
	10:00					0	0	0	-	0		0	0			0	-	0
10:00 -						0	0	0	-	0	-	0	0	0	-	0	-	0
10:00 -						0	0	0	-	0		0	0			0	-	0
10:13 -						0	0	0	-	0		0	0	0		0		0
10:30 -						0	0	0	0	0		0	0	0		0	-	0
11:00 -						0	0	0		0	-	0	0	0	-	0	-	0
11:15 -						0	0	0	0	0	-	0	0	0	-	0	-	0
11:30 -						0	0	0	-	0		0	0	0		0	-	0
11:45 -						0	0	0	-	0		0	0	0		0	-	0
12:00 -						0	0	0	-	0		0	0			0	-	0
12:15 -						0	0	0	-	0		0	0			0		0
12:30 -						0	0	0	-	0		0	0			0	-	0
12:45 -						0	0	0	-	0		0	0	0		0		0
1:00 -						0	0	0	-	0		0	0			0	-	0
1:15 -						0	0	0	-	0		0	0	0		0		0
1:30 -						ő	0	0	0	0		0	0	ő		0	-	Ő
1:45 -		(0	0	0		0		0	0	o 0		0	-	0
2:00 -		(Ö	0	0	0	0	-	0	0	Ö		0	-	0
2:15 -		(Ö	0	0	-	0		0	0	Ö		0	-	0
2:30 -		(Ö	0	0	-	0		0	0			0		0
2:45 -		(0	0	0	-	0		0	0			0		0
3:00 -		(Ö	0	0	0	0		0	0			0		0
3:15 -		(Ö	0	0	-	0		0	0			0	-	0
3:30 -		() () (0	0	0	0		0	0	0	0	0	0	0	0	0
3:45 -		() () (0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 -	4:15	() () (0 0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 -	4:30	() 11	1 (0 0	2	18	0	0	0	0	0	0	0	0	5	0	36
4:30 -	4:45	() 14	1 2	2 0	2	16	0	0	0	0	0	0	0	0	9	0	43
4:45 -		() 11	1 (0 0	1	11	0	0	0	0	0	0	0	0	5	0	28
5:00 -	5:15	() 17	7 1	1 0	9	13	0	0	0	0	0	0	0	0	8	0	48
5:15 -		() 13	3 (0 0	9	14	0	0	0	0	0	0	2	0	14	0	52
5:30 -		(7	21	0		0		0	0			3		41
5:45 -	6:00	() () (0 (0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 -	6:15	() () (0 (0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 -		() () (0 (0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 -	6:45	() () (0 (0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 -	7:00	() () (0 0	0	0	0	0	0	0	0	0	0	0	0	0	0
		() 170) 9	9 0	72	130	0	0	0	0	0	0	6	0	79	0	466

Abelin Traffic Services

130 S. Howie Street Helena, MT 59601

File Name: 282RunkleT

Site Code : 00000000 Start Date : 2/23/2021

Page No : 1

	282 RUNKLE																						
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			uthbo	und				estbo	und				orthbo	und				astbou	<u>ınd</u>				
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total		
07:30 AM	1	41	0	0	42	0	0	0	0	0	0	7	1	0	8	6	0	5	0	11	61		
07:45 AM	3	46	0	0	49	0	0	0	0	0	0	23	0	0	23	14	0	10	0	24	96		
Total	4	87	0	0	91	0	0	0	0	0	0	30	1	0	31	20	0	15	0	35	157		
08:00 AM	4	45	0	0	49	0	0	0	0	0	0	7	5	0	12	9	0	7	0	16	77		
08:15 AM	0	16	0	0	16	0	0	0	0	0	0	7	4	0	11	8	0	1	0	9	36		
*** BREAK *	**																				'		
Total	4	61	0	0	65	0	0	0	0	0	0	14	9	0	23	17	0	8	0	25	113		
*** BREAK *	**																						
04:30 PM	3	13	0	0	16	0	0	0	0	0	0	36	3	0	39	8	0	2	0	10	65		
04:45 PM	7	12	0	0	19	0	0	0	0	0	0	25	6	0	31	4	0	5	0	9	59		
Total	10	25	0	0	35	0	0	0	0	0	0	61	9	0	70	12	0	7	0	19	124		
05:00 PM	3	7	0	0	10	0	0	0	0	0	0	42	15	0	57	7	0	1	0	8	75		
05:15 PM	3	10	0	0	13	0	0	0	0	0	0	35	15	0	50	1	0	2	0	3	66		
*** BREAK *	**																						
Total	6	17	0	0	23	0	0	0	0	0	0	77	30	0	107	8	0	3	0	11	141		
*** BREAK *	**																						
Grand Total	24	190	0	0	214	0	0	0	0	0	0	182	49	0	231	57	0	33	0	90	535		
Apprch %	11.2	88.8	0	0		0	0	0	0		0	78.8	21.2	0		63.3	0	36.7	0				
Total %	4.5	35.5	0	0	40	0	0	0	0	0	0	34	9.2	0	43.2	10.7	0	6.2	0	16.8			
Unshifted	24	190	0	0	214	0	0	0	0	0	0	182	49	0	231	57	0	33	0	90	535		
% Unshifted																							
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
% Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		

Abelin Traffic Services 130 S. Howie Street

Helena, MT 59601

File Name: RunkleAlpineTMC

Site Code : 00000000 Start Date : 2/24/2021

Page No : 1

	Groups Printed- Unshifted - Bank 2 ALPINE VEIW RUNKLE ALPINE VEIW RUNKLE																				
	Д	LPINE	VEIV	٧			RUNK	LE			Δ	LPINE	E VEI\	V							
		So	uthbo	und			W	estbo	und			No	orthbo	und			E	astbo	und		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
*** BREAK *	**																				
07:30 AM	0	0	0	0	0	0	1	1	0	2	1	1	1	0	3	0	10	0	0	10	15
07:45 AM	0	0	0	0	0	0	4	0	0	4	3	0	1	0	4	0	21	0	0	21	29
Total	0	0	0	0	0	0	5	1	0	6	4	1	2	0	7	0	31	0	0	31	44
																				•	
08:00 AM	0	0	0	0	0	0	8	0	0	8	1	0	0	0	1	0	15	0	0	15	24
08:15 AM	0	0	0	0	0	0	4	1	0	5	1	0	1	0	2	6	8	0	0	14	21
*** BREAK *	**					•														•	
Total	0	0	0	0	0	0	12	1	0	13	2	0	1	0	3	6	23	0	0	29	45
*** BREAK *	**																				
04:30 PM	0	0	0	0	0	0	5	1	0	6	2	0	1	2	5	0	6	0	0	6	17
04:45 PM	0	0	0	0	0	0	13	2	1	16	1	0	0	0	1	1	9	0	1	11	28
Total	0	0	0	0	0	0	18	3	1	22	3	0	1	2	6	1	15	0	1	17	45
05:00 PM	0	0	0	0	0	0	15	3	0	18	2	0	1	0	3	0	3	2	0	5	26
05:15 PM	0	0	0	0	0	0	12	2	0	14	0	0	0	0	0	0	4	0	0	4	18
*** BREAK *	**																				
Grand Total	0	0	0	0	0	0	62	10	1	73	11	1	5	2	19	7	76	2	1	86	178
Apprch %	0	0	0	0		0	84.9	13.7	1.4		57.9	5.3	26.3	10.5		8.1	88.4	2.3	1.2		
Total %	0	0	0	0	0	0	34.8	5.6	0.6	41	6.2	0.6	2.8	1.1	10.7	3.9	42.7	1.1	0.6	48.3	
Unshifted	0	0	0	0	0	0	62	10	1	73	11	1	5	2	19	7	76	2	1	86	178
% Unshifted	0	0	0	0	0	0	100	100	100	100	100	100	100	100	100	100	100	100	100	100	100
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
, , , , , , , , , , , , , , , , , , , ,	, ,	•	•	•	Ū	, ,	•	•	•	J	, ,	•	•	•	•		•	-	•	5	

Turning Movement Count All Vehicles

Location Highway 287 & Crossroads Parkway
Date 11/3/2022

Date	11/3/2022																	
			Northb					bound			Eastbo				Westbo		.	
- 00		Left			Peds		Thr		Peds		Thr		Peds				Peds	
7:00 -		21	0	2			1		0	0	_			4	238	6	0	363
7:15 -		27		1	0	5	(-	0	2			0	0	201	0	0	324
7:30 -		39		4	0	5	() 1	0	2				4	345	4	0	533
7:45 -		29		0	0	/	1	1 2	0	1	102	17	0	7	275	3	0	445
8:00 -		16	0	1	0	4	2		0	0	89	15	0	2	249	1	0	383
8:15 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
8:30 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
8:45 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
9:00 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
9:15 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
9:30 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
9:45 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
10:00 -		0	0	0	0	0		0	0	0	0	-	0	0	0	0	0	0
10:15 -		0	-	0	0	0		0	0	0	•	-	0	0	0	0	0	0
10:30 -		0	0	0	0	0	(-	0	0	0	-	0	0	0	0	0	0
10:45 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
11:00 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
11:15 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
11:30 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
11:45 -		0	0	0	0	0	(0	0	0	•	0	0	0	0	0	0
12:00 -	-	0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
12:15 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
12:30 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
12:45 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
1:00 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
1:15 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
1:30 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
1:45 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
2:00 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
2:15 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
2:30 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
2:45 - 3:00 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
3:00 - 3:15 -		0	0	0	0	0	(-	0	0	0	0	0	0	0	0	0	0
3:30 -		•	-	-				-	-	•	-	-		-	-	-		
3:45 -		0	0	0	0		(0 0	0			0	0	0	0	0	0
4:00 -		0		0	0		(0	0	_	0	0 0	0	0	0	0	0
4:00 - 4:15 -		23		4	0	7	1		0	1	245	25	0	2	139	4	0	453
4:30 -		23		7	0	8	1		0	2		14	0	4	140	2	0	444
4:45 -		25 25		4	0	8	(0	2			0	2	130	1	0	451
5:00 -		24		2	0	13	(0	0		26	0	2	131	3	0	473
5:15 -		21	1	1	0	10	(0	1	310		0	2	152	3	0	524
5:30 -		19	0	1	0	5	1		0	0	274	20	0	2	129	2	0	455
5:45 -		0	0	0	0	0	C		0	0	0	0	0	0	0	0	0	733
6:00 -		0	0	0	0	0	(0	0	0	0	0	0	0	0	0	0
6:15 -		0	0	0	0	0	(0	0	-	0	0	0	0	0	0	0
6:30 -		0	0	0	0	0	(0	0	•	_	0	0	0	0	0	0
6:45 -		0	_	0	0	0	(0	0	•	-	0	0	0	0	0	ő
0.40 -		267		27	0	74	7		0	11	2057	194	0	31	2129	29	0	4848
		201	J	۲ ا	J	17	,	. ,	J		2001	10-4	J	01	2120	23	0	-0-0

For Project:	AliceStreet Helena							
Project Notes:								
Location/Name:	Merged							
Report Generated:	8/3/2022	09:06						
Speed Intervals	1 MPH							
Time Intervals	Instant							
Traffic Report From	7/19/2022	10:00:00	through	7/21/2022	11:59:59			
85th Percentile Speed	37 MPH							
85th Percentile Vehicles	3703							
Max Speed	67 MPH	on	7/19/2022	17:26:37				
Total Vehicles	4357							
AADT:	2091							
Volumes -								
weekly counts								
	Time	5 Day	7 Day					
Average Daily		1452	1452					
AM Peak	07:00	186	186					
PM Peak	12:00	196	196					
Speed								
Speed Limit:	35							
85th Percentile Speed:	37							
50th Percentile Speed:	31							
10 MPH Pace Interval:	28.0 MPH	to	38.0 MPH					
Average Speed:	29.54							
	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	
Count over limit	N/A	324	502	174	N/A	N/A	N/A	
% over limit	N/A	22.7	23.2	22.7	N/A	N/A	N/A	
Avg Speeder	N/A	38.5	38.6	38.6	N/A	N/A	N/A	
Class Counts								
	Number		%					
VEH_SM	101		2.3					
VEH_MED	4107		94.3					
VEH_LG	149		3.4					

	7/18/2022	to	7/24/2022							
	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Week	Weekend	Week Day 85%
Hour	7/18/2022	7/19/2022	7/20/2022	7/21/2022	7/22/2022	7/23/2022	7/24/2022	Day Avg	Avg	Avg Speed
0 - 1	*	*	4	3	*	*	*	3.5	0	33.5
1 - 2	*	*	2	1	*	*	*	1.5	0	31.5
2 - 3	*	*	1	2	*	*	*	1.5	0	32
3 - 4	*	*	5	4	*	*	*	4.5	0	36
4 - 5	*	*	4	4	*	*	*	4	0	30
5 - 6	*	*	36	38	*	*	*	37	0	35.9
6 - 7	*	*	71	51	*	*	*	61	0	37.25
7 - 8	*	*	191	182	*	*	*	186.5	0	36.3
8 - 9	*	*	164	137	*	*	*	150.5	0	36.2
9 - 10	*	*	116	113	*	*	*	114.5	0	36.65
10 - 11	*	18	151	109	*	*	*	92.67	0	35.13
11 - 12	*	165	149	122	*	*	*	145.33	0	36.37
12 - 13	*	177	215	*	*	*	*	196	0	36.35
13 - 14	*	127	160	*	*	*	*	143.5	0	36.7
14 - 15	*	112	122	*	*	*	*	117	0	36.7
15 - 16	*	148	115	*	*	*	*	131.5	0	35.75
16 - 17	*	158	189	*	*	*	*	173.5	0	36.75
17 - 18	*	221	166	*	*	*	*	193.5	0	36.95
18 - 19	*	90	91	*	*	*	*	90.5	0	37.3
19 - 20	*	81	65	*	*	*	*	73	0	37.65
20 - 21	*	72	45	*	*	*	*	58.5	0	36.5
21 - 22	*	39	61	*	*	*	*	50	0	35.5
22 - 23	*	21	29	*	*	*	*	25	0	35
23 - 24	*	1	9	*	*	*	*	5	0	32.5
Totals	0	1430	2161	766	0	0	0			
% of Total	0%	32.82%	49.6%	17.58%	0%	0%	0%			
										Daga 1

Page 1

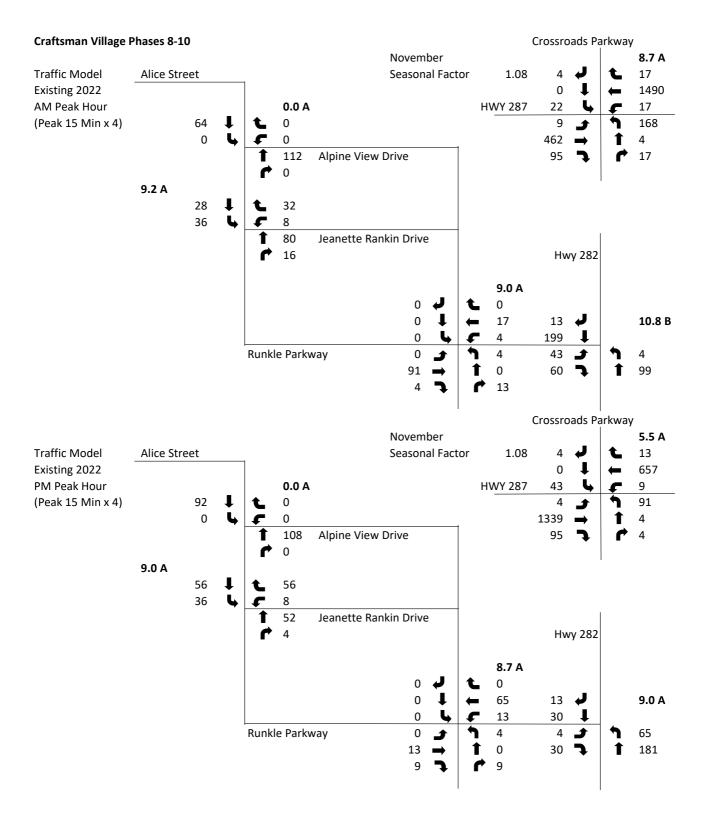
For Project:	Jeannette Rankin Helena	a						
Project Notes:								
Location/Name:	Merged							
Report Generated:	8/3/2022	09:12						
Speed Intervals	1 MPH							
Time Intervals	Instant			- 1 1				
Traffic Report From	7/19/2022	10:00:00	through	7/21/2022	11:59:59			
85th Percentile Speed	28 MPH							
85th Percentile Vehicles	839							
Max Speed	43 MPH	on	7/20/2022	11:29:54				
Total Vehicles	987							
AADT:	473							
Volumes -								
weekly counts								
	Time	5 Day	7 Day					
Average Daily		329	329					
AM Peak	08:00	48	48					
PM Peak	12:00	51	51					
Speed								
Speed Limit:	35							
85th Percentile Speed:	28							
50th Percentile Speed:	23							
10 MPH Pace Interval:	18.0 MPH	to	28.0 MPH					
Average Speed:	23.04							
	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	
Count over limit	N/A	3	10	5	N/A	N/A	N/A	
% over limit	N/A	0.9	2.1	2.7	N/A	N/A	N/A	
Avg Speeder	N/A	38.0	38.4	40.6	N/A	N/A	N/A	
Class Counts								
	Number		%					
VEH_SM	70		7.1					
VEH_MED	883		89.5					
VEH_LG	34		3.4					
[VEH_SM=motorcycle,	VEH_MED = sedan,		VEH_LG = truck]					

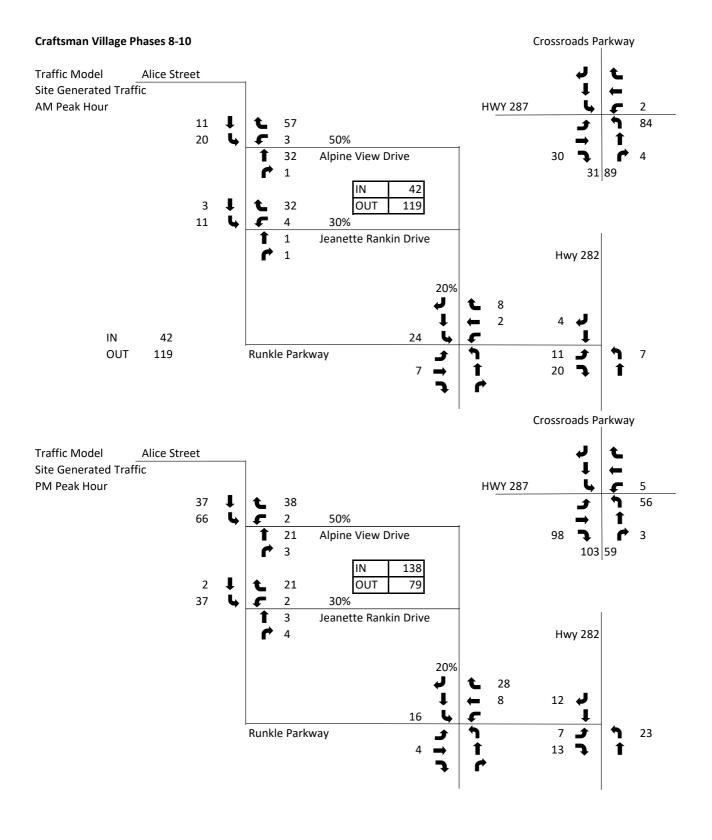
	7/18/2022	to	7/24/2022							
	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Week	Weekend	Week Day 85%
Hour	7/18/2022	7/19/2022	7/20/2022	7/21/2022	7/22/2022	7/23/2022	7/24/2022	Day Avg	Avg	Avg Speed
0 - 1	*	*	1	0	*	*	*	0.5	0	21
1 - 2	*	*	0	0	*	*	*	0	0	0
2 - 3	*	*	0	0	*	*	*	0	0	0
3 - 4	*	*	0	1	*	*	*	0.5	0	23
4 - 5	*	*	1	1	*	*	*	1	0	20.5
5 - 6	*	*	8	8	*	*	*	8	0	30
6 - 7	*	*	15	12	*	*	*	13.5	0	27.5
7 - 8	*	*	45	47	*	*	*	46	0	28.6
8 - 9	*	*	51	45	*	*	*	48	0	28.5
9 - 10	*	*	21	24	*	*	*	22.5	0	27
10 - 11	*	9	27	24	*	*	*	20	0	26.33
11 - 12	*	51	38	21	*	*	*	36.67	0	25.93
12 - 13	*	48	54	*	*	*	*	51	0	27.1
13 - 14	*	32	42	*	*	*	*	37	0	26.75
14 - 15	*	18	25	*	*	*	*	21.5	0	25.75
15 - 16	*	31	28	*	*	*	*	29.5	0	28.75
16 - 17	*	31	40	*	*	*	*	35.5	0	28.5
17 - 18	*	48	24	*	*	*	*	36	0	27.1
18 - 19	*	11	20	*	*	*	*	15.5	0	29.75
19 - 20	*	15	10	*	*	*	*	12.5	0	29
20 - 21	*	13	10	*	*	*	*	11.5	0	27
21 - 22	*	10	14	*	*	*	*	12	0	28
22 - 23	*	4	7	*	*	*	*	5.5	0	25.75
23 - 24	*	0	2	*	*	*	*	1	0	24
Totals	0	321	483	183	0	0	0			
% of Total	0%	32.52%	48.94%	18.54%	0%	0%	0%			
										Daga 1

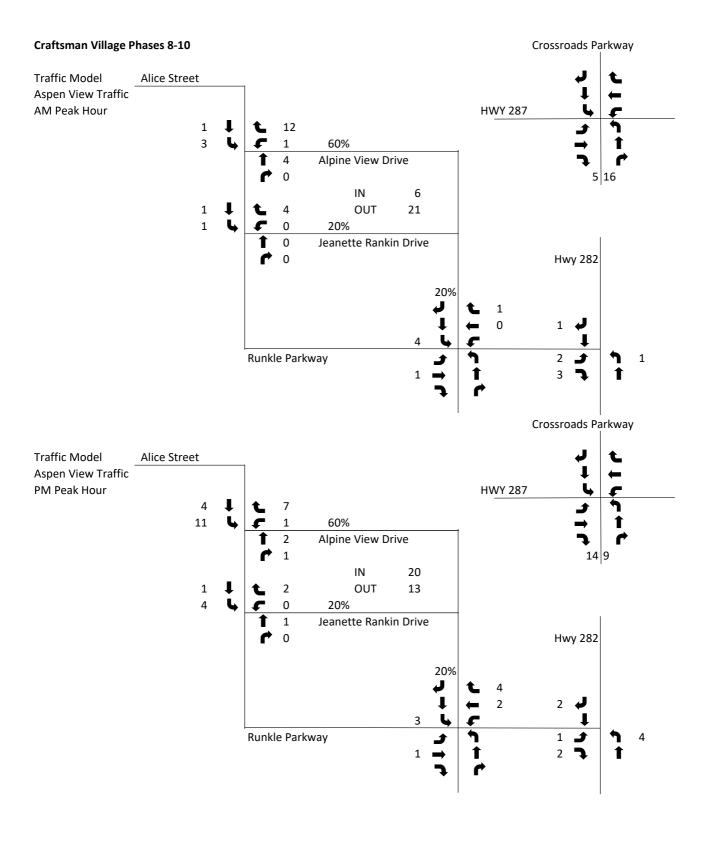
Page 1

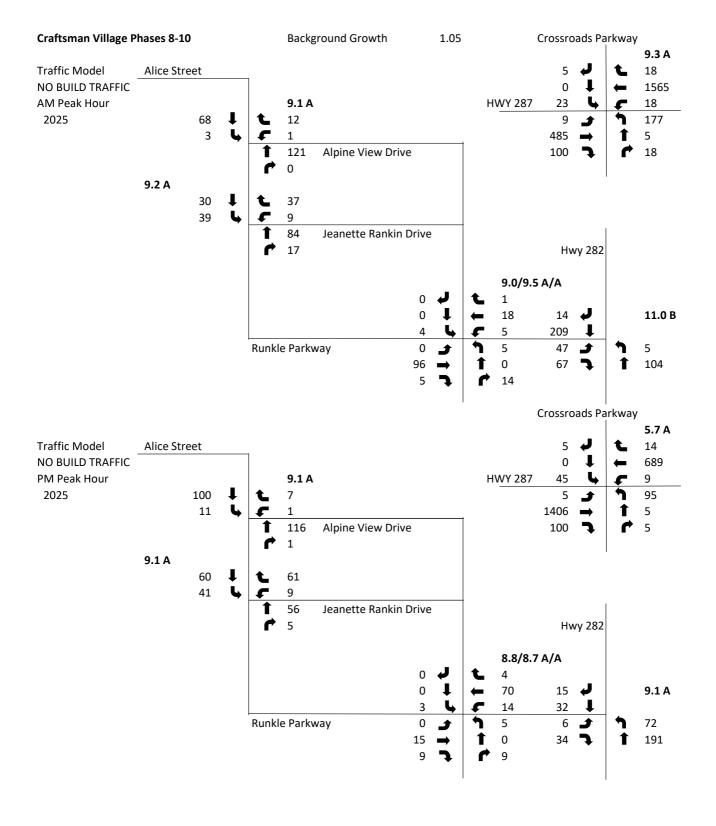
APPENDIX B

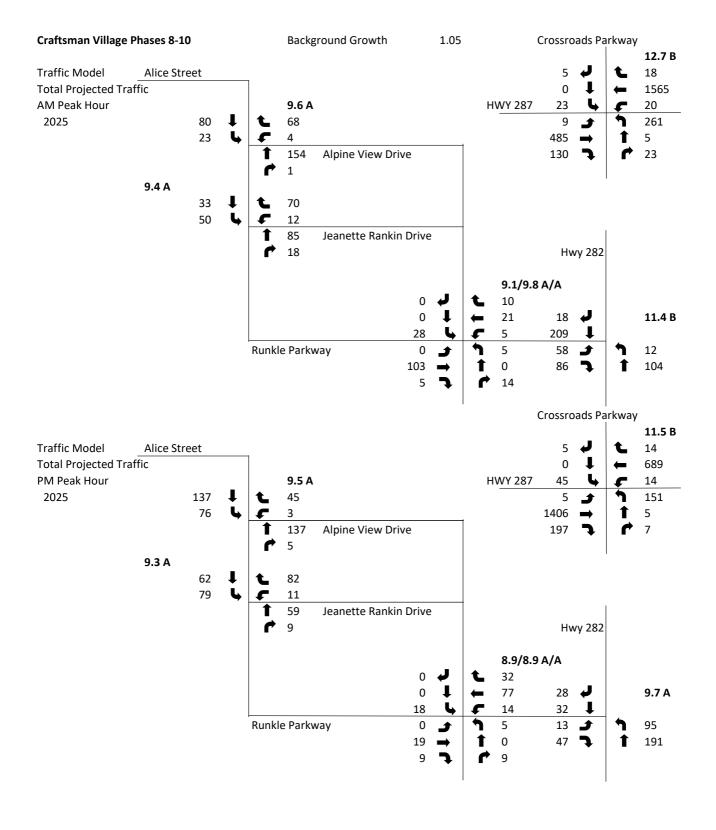
Traffic Model











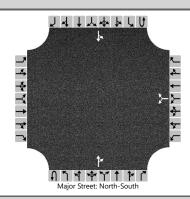
APPENDIX C

LOS Calculations

		HCS	Sigr	nalize	d Inte	ersect	ion R	esu	lts Su	nmary	/				
													ļ.,		
General Inforn	nation									ction Inf	_			.↓. ↓. ↓.	ba l _a
Agency		ATS							Duratio	·	0.250)		*	P.
Analyst		RLA				e Nov 3	, 2022		Area Ty	ре	Other	r	<i>3</i> →		<u>≛</u> <u>4</u> —5
Jurisdiction		MDT		Time F			ng AM F	Peak	PHF		1.00		4-4	w‡e s	← ∳ ←
Urban Street		HWY 287		Analys	is Yea	r 2022			Analysi	Period	1> 7:	00	→		* €
Intersection		Crossroads Parkwy		File Na	ame	HWY:	287AM.:	xus						ን ጅ	
Project Descrip	tion	MVM											1	1 *1 *1 *** *** *1	7 4
Demand Inform	mation				EB			W	В		NB		7	SB	
Approach Move	ement			L	Т	R	L	T	R	L	Тт	R	L	Т	R
Demand (v), v				9	462		17	14	_	168	4	17	22	0	4
2011101112 (17), 1															
Signal Informa	ation						T	\top							T
Cycle, s	90.0	Reference Phase	2		₩ 1	T 84	2						$\boldsymbol{\leftrightarrow}$	1	stz i
Offset, s	0	Reference Point	End	Green	69.0	11.0	0.0	0.0	0.0	0.0		1	M 2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.0	0.0	0.0					→		KÎZ
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0	0.0	0.0		5	6	7	8
Times Deculto				EDI	_	EDT	WD		WDT	ND		NDT	CDI		CDT
Timer Results				EBI	-	EBT	WB	-	WBT	NB	_	NBT	SBI	-	SBT
Assigned Phas	e			_	_	2		-	6	-		8		_	4
Case Number				_	-	5.0	_	-	6.0	-		6.0	-	+	8.0
Phase Duration		,			_	74.0	_	-	74.0	₩		16.0	_	\rightarrow	16.0
Change Period		·			-	5.0	-	-	5.0	-	_	5.0	-	-	5.0
Max Allow Head		·		_	_	0.0	_	_	0.0	-	_	3.1	_	\rightarrow	3.1
Queue Clearan		, = ,			-			-		-	_	13.0	_		4.0
Green Extension		(g e), s		_	_	0.0	_	-	0.0	-	_	0.0	-	+	0.2
Phase Call Pro					-			-		-	-	1.00			1.00
Max Out Proba	bility		_		_	_		_	_		_	1.00		_	0.02
Movement Gro	oup Res	sults			EB			WE	3		NB			SB	
Approach Move				L	Т	R	L	Т	R		Т	R	L	Т	R
Assigned Move				5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow I		'), veh/h		9	462	95	17	755	752	168	21			26	
		ow Rate (s), veh/h/l	n	367	1809	_	983	190	_	1492	1658			1423	
Queue Service		, ,		0.9	3.1	1.3	0.4	13.8		9.0	1.0			0.9	
	•	e Time (<i>g c</i>), s		14.7	3.1	1.3	3.5	13.8	3 13.9	11.0	1.0	1		2.0	
Green Ratio (g	r/C)	, ,		0.77	0.77	0.77	0.77	0.7	7 0.77	0.12	0.12			0.12	
Capacity (c), v	/eh/h			305	2773	1284	800	145	7 1451	230	203			248	
Volume-to-Cap	acity Ra	atio (X)		0.030	0.167	0.074	0.021	0.51	8 0.519	0.730	0.104	i –		0.105	
Back of Queue	(Q), f	t/ln (95 th percentile)	3.4	34.6	14	3.3	176.	8 176.5	192.5	18.2			22.8	
Back of Queue	(Q), v	eh/ln (95 th percenti	le)	0.1	1.4	0.6	0.1	7.1	7.1	7.7	0.7			0.9	
		RQ) (95 th percent		0.00	0.00	0.00	0.00	0.00		0.00	0.00			0.00	
Uniform Delay				6.9	2.8	2.6	3.3	4.1	4.1	41.0	35.1			35.6	
Incremental De				0.2	0.1	0.1	0.0	1.3	1.3	9.9	0.1			0.1	
Initial Queue De	- '	·		0.0	0.0	0.0	0.0	0.0	_	0.0	0.0			0.0	
Control Delay (d), s/v	eh		7.1	2.9	2.7	3.3	5.4	5.4	50.8	35.2			35.7	
Level of Service	e (LOS)			Α	Α	А	Α	А	А	D	D			D	
Approach Delay	y, s/veh	/LOS		3.0		Α	5.4		Α	49.	1	D	35.7	,	D
Intersection De						8	.7						Α		
Mariation 1.1.5								10/5			ND			65	
Multimodal Re		/1.00		4.00	EB		4.0	WE		0.0	NB		0.45	SB	D
Pedestrian LOS				1.83		В	1.6	_	В	2.3		В	2.45	_	В
Bicycle LOS So	core / L0	JS		0.95		Α	1.74	7	В	0.8	J	Α	0.53	,	Α

		HCS	S Sigr	nalize	d Int	ersect	ion R	esu	lts Sur	nmary	/				
													ļ.,		
General Inform	nation								Intersec		_			.↓. ↓. ↓.	ba l _a
Agency		ATS							Duration	·	0.250			*	P.
Analyst		RLA				e Nov 3	, 2022		Area Ty	ре	Other	-	<u></u>		<u>≛</u> <u>4</u> —5
Jurisdiction		MDT		Time F			ng PM F	Peak	PHF		1.00		-₹ -₹	w‡e s	← ∳ ←
Urban Street		HWY 287				r 2022			Analysis	Period	1> 7:	00	→		* €
Intersection		Crossroads Parkwy	'	File Na	ame	HWY:	287PM.:	xus						ን ጅ	
Project Descrip	tion	MVM											*	1 *1 *1 *** *** *1	7 4
Demand Inform	nation			Γ	EB			V	/B	7	NB		7	SB	
Approach Move	ement			L	Т	R	L	Т-	Г В	L	Т	R	L	Т	R
Demand (v), v				4	1339	_	9	6	_	91	4	4	43	0	4
2011101112 (17), 1	J.,,								,						
Signal Informa	tion						T	Т		\top					T
Cycle, s	90.0	Reference Phase	2		層	T 84	2						\Leftrightarrow	l l	SŢZ
Offset, s	0	Reference Point	End	Green	73.0	7.0	0.0	0.0	0.0	0.0		1	Y 2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.0	0.0	0.0		0.0			→		ĸtz
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0		0.0		5	- 6	7	8
					-	EDT	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		MART	l un		NDT	0.01	-	ODT
Timer Results				EBI	-	EBT	WB	L	WBT	NB	L	NBT	SBI		SBT
Assigned Phase	e				_	2	_	-	6	₩	_	8	-	\rightarrow	4
Case Number					-	5.0	_	-	6.0	-	_	6.0	-	+	8.0
Phase Duration					_	78.0		-	78.0	_	_	12.0	_	\rightarrow	12.0
Change Period		,			_	5.0		-	5.0	-	_	5.0	-	_	5.0
Max Allow Head		· · · · · · · · · · · · · · · · · · ·			_	0.0		_	0.0			3.1	_	\rightarrow	3.1
Queue Clearan		, - ,			_			_		-	_	9.0	_	\rightarrow	4.8
Green Extension		(g e), s			_	0.0	_	_	0.0			0.0		\rightarrow	0.0
Phase Call Pro					_		_	_		-		0.97	_		0.97
Max Out Proba	bility		_		_			_				1.00		_	1.00
Movement Gro	oup Res	sults			EB			WE	3		NB			SB	
Approach Move				1	T	R	L	T	R	L	T	R	L	T	R
Assigned Move				5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow I		'), veh/h		4	1339	+	9	336	_	91	8	1.5		47	
		ow Rate (s), veh/h/l	n	811	1809	+	431	190	_	1492	1743			1471	
Queue Service			••	0.1	10.0		0.6	3.7		4.2	0.4			2.4	
		e Time (<i>g</i> ε), s		3.8	10.0	1.0	10.6	3.7		7.0	0.4			2.8	
Green Ratio (g		(90),0		0.81	0.81	0.81	0.81	0.8		0.08	0.08			0.08	
Capacity (c), v				705	2934		382	154		149	136			191	
Volume-to-Capa		atio (X)		0.006			0.024	0.21			0.059			0.246	
		t/ln (95 th percentile	:)	0.6	86.4	8.9	2.4	35.	_	99.2	7.3			44.2	
	, ,	eh/ln (95 th percenti		0.0	3.5	0.4	0.1	1.4		4.0	0.3			1.8	
		RQ) (95 th percent		0.00	0.00	0.00	0.00	0.0		0.00	0.00			0.00	
Uniform Delay (,	2.4	2.5	1.7	4.1	2.0	_	43.4	38.4			39.6	
Incremental De	`			0.0	0.5	0.1	0.1	0.3	_	5.2	0.1			0.2	
Initial Queue De				0.0	0.0	0.0	0.0	0.0		0.0	0.0			0.0	
Control Delay (<u>, </u>		2.4	3.1	1.8	4.2	2.3	_	48.6	38.5			39.9	
Level of Service				A	A	A	A	A	A	D	D			D	
Approach Delay				3.0		A	2.3		A	47.8		D	39.9		D
Intersection De				5.5			5.5		• •				Α		
Multimodal Re					EB			WE			NB			SB	
Pedestrian LOS				1.82		В	1.59	_	В	2.30		В	2.46	_	В
Bicycle LOS Sc	ore / LO	OS		1.67	7	В	1.0	5	Α	0.6	5	Α	0.57	'	Α

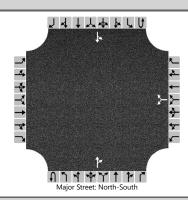
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Alpine View
Analysis Year	2022	North/South Street	Alice
Time Analyzed	AM peak existing	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						0		0			112	0		0	64	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)		0														
Right Turn Channelized																
Median Type Storage		Undivided														
Critical and Follow-up He	adwa															
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)							0							0		
Capacity, c (veh/h)							0							1459		
v/c Ratio														0.00		
95% Queue Length, Q ₉₅ (veh)														0.0		
Control Delay (s/veh)														7.5	0.0	
Level of Service (LOS)														А	А	
Approach Delay (s/veh)													0.0			
Approach LOS													A			

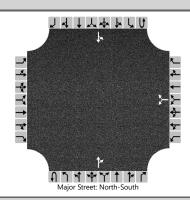
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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Alpine View
Analysis Year	2022	North/South Street	Alice
Time Analyzed	PM peak existing	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



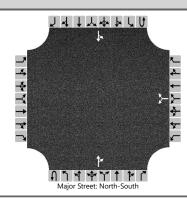
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						0		0			108	0		0	92	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)		0														
Right Turn Channelized																
Median Type Storage		Undivided														
Critical and Follow-up He	eadwa															
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)							0							0		
Capacity, c (veh/h)							0							1465		
v/c Ratio														0.00		
95% Queue Length, Q ₉₅ (veh)														0.0		
Control Delay (s/veh)														7.5	0.0	
Level of Service (LOS)														А	А	
Approach Delay (s/veh)											0.0					
Approach LOS													А			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Rankin
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Jeannette Rankin
Analysis Year	2022	North/South Street	Alice
Time Analyzed	AM peak existing	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



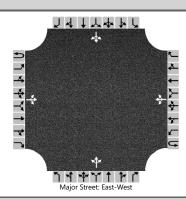
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						8		32			80	16		36	28	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						(0									
Right Turn Channelized																
Median Type Storage		Undivided														
Critical and Follow-up He	eadwa															
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)							43							39		
Capacity, c (veh/h)							911							1481		
v/c Ratio							0.05							0.03		
95% Queue Length, Q ₉₅ (veh)							0.2							0.1		
Control Delay (s/veh)							9.2							7.5	0.2	
Level of Service (LOS)							А						A A			
Approach Delay (s/veh)		9.2											4.3			
Approach LOS						,	Α						А			

HCS Two-Way Stop-Control Report											
General Information		Site Information									
Analyst	RLA	Intersection	Alice and Rankin								
Agency/Co.	ATS	Jurisdiction	Lewis and Clark								
Date Performed	8/2/2022	East/West Street	Jeannette Rankin								
Analysis Year	2022	North/South Street	Alice								
Time Analyzed	PM peak existing	Peak Hour Factor	0.92								
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25								
Project Description	Mountain View										



Approach	Eastbound				Westbound				Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0	
Configuration							LR					TR		LT			
Volume (veh/h)						8		56			52	4		36	56		
Percent Heavy Vehicles (%)						3		3						3			
Proportion Time Blocked																	
Percent Grade (%)						()										
Right Turn Channelized																	
Median Type Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)						7.1		6.2						4.1			
Critical Headway (sec)						6.43		6.23						4.13			
Base Follow-Up Headway (sec)						3.5		3.3						2.2			
Follow-Up Headway (sec)						3.53		3.33						2.23			
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)							70							39			
Capacity, c (veh/h)							967							1536			
v/c Ratio							0.07							0.03			
95% Queue Length, Q ₉₅ (veh)							0.2							0.1			
Control Delay (s/veh)							9.0							7.4	0.2		
Level of Service (LOS)							А							А	Α		
Approach Delay (s/veh)						9.0								3.0			
Approach LOS						А							A				

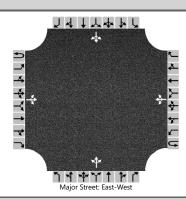
HCS Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	RLA	Intersection	Rundle and Alpine View							
Agency/Co.	ATS	Jurisdiction	Lewis and Clark							
Date Performed	8/2/2022	East/West Street	Rundle							
Analysis Year	2022	North/South Street	Alpine							
Time Analyzed	AM peak existing	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	Mountain View									



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound		Westbound				Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0	
Configuration			LTR				LTR				LTR				LTR		
Volume (veh/h)		0	91	4		4	17	0		4	0	13		0	0	0	
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3	
Proportion Time Blocked																	
Percent Grade (%)									0				0				
Right Turn Channelized																	
Median Type Storage		Undivided															
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33	
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)		0				4					18				0		
Capacity, c (veh/h)		1592				1482					923				0		
v/c Ratio		0.00				0.00					0.02						
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.1						
Control Delay (s/veh)		7.3	0.0	0.0		7.4	0.0	0.0			9.0						
Level of Service (LOS)		Α	А	А		Α	Α	Α			Α						
Approach Delay (s/veh)	0.0				1.4				9.0								
Approach LOS	А				А				,	4							

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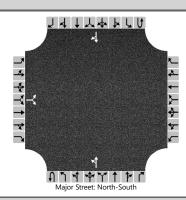
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2022	North/South Street	Alpine
Time Analyzed	PM peak existing	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		0	13	9		13	65	0		4	0	9		0	0	0
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										(0			(0	
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up He	eadwa	ndways														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				14					14				0	
Capacity, c (veh/h)		1523				1584					982				0	
v/c Ratio		0.00				0.01					0.01					
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.0					
Control Delay (s/veh)		7.4	0.0	0.0		7.3	0.1	0.1			8.7					
Level of Service (LOS)		А	Α	Α	A A A A			А								
Approach Delay (s/veh)	0.0 1.3								8.7							
Approach LOS	A A							A								

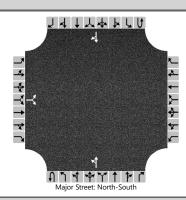
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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Hwy 282
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2022	North/South Street	Hwy 282
Time Analyzed	AM peak existing	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume (veh/h)		43		60						4	99				199	13
Percent Heavy Vehicles (%)		3		3						3						
Proportion Time Blocked																
Percent Grade (%)		(0													
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		6.43		6.23						4.13						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)			112							4						
Capacity, c (veh/h)			737							1332						
v/c Ratio			0.15							0.00						
95% Queue Length, Q ₉₅ (veh)			0.5		Ì				Ì	0.0						
Control Delay (s/veh)			10.8							7.7	0.0					
Level of Service (LOS)			В							А	А					
Approach Delay (s/veh)	10.8								0.3							
Approach LOS	В								А							

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Hwy 282
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2022	North/South Street	Hwy 282
Time Analyzed	PM peak existing	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		

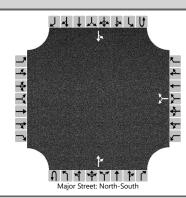


Vehicle Volumes and Adju	stme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume (veh/h)		4		30						65	181				30	13
Percent Heavy Vehicles (%)		3		3						3						
Proportion Time Blocked																
Percent Grade (%)		(0													
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		6.43		6.23						4.13						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)			37							71						
Capacity, c (veh/h)			946							1554						
v/c Ratio			0.04							0.05						
95% Queue Length, Q ₉₅ (veh)			0.1						Ì	0.1						
Control Delay (s/veh)			9.0							7.4	0.4					
Level of Service (LOS)			А							А						
Approach Delay (s/veh)	9.0															
Approach LOS	A							А								

		нся	Sigr	nalize	d Inte	ersect	ion R	esu	lts Sun	nmary	•				
General Informa	ation								Intersec	tion Inf	ormatio	nn.		4 가하 t	يا مل
Agency	ation	ATS							Duration		0.250			4	
Analyst		RLA		Analys	sic Date	e Nov 3	2022	_	Area Typ		Other				Ł.
Jurisdiction		MDT		Time F			ild AM I	Pook	PHF	,e	1.00			w∳E	<u>~</u> }- ← ÷
Urban Street		HWY 287				r 2025	IIIU AIVI I	eak	Analysis	Poriod	1> 7:0	20			, _ €
Intersection		Crossroads Parkwy	,	File Na			287AMn	obuile		Periou	177.	00			
Project Description	on	MVM		File IN	ame		20 <i>1</i> AIVII I	Obuild	ı.xus				-	<u>ነ</u> ነ ተተቀጥ	^۳ ۳
				_			_		_	_			_		
Demand Inform					EB		-	W		-	NB		-	SB	
Approach Mover				L	T	R	L	I		L	T	R	L	T	R
Demand (v), ve	eh/h		-	9	485	100	18	15	65 18	177	5	18	23	0	5
Signal Informat	ion				<u>.</u> ا		Т	Т							1
Cycle, s	90.0	Reference Phase	2		肾		a						\Leftrightarrow		stz
Offset, s	0	Reference Point	End	Green	69.0	11.0	0.0	0.0	0.0	0.0		1	Y 2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.0	0.0	0.0		0.0			→		松
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0		0.0		5	6	7	8
Times Decults				EDI		CDT	\\/D		\A/DT	NDI	7	NDT	CDI		CDT
Timer Results				EBI	-	EBT 2	WB	-	WBT	NBI	-	NBT	SBI	-	SBT
Assigned Phase				_			_	\rightarrow	6			8		_	4
Case Number				_	-	5.0	_	-	6.0		-	6.0		-	8.0
Phase Duration,		\ -		_	_	74.0	_	-	74.0			16.0		_	16.0
Change Period,				_	-	5.0	-	-	5.0		-	5.0			5.0
	x Allow Headway (<i>MAH</i>), s			_		0.0	-	-	0.0	_		3.1	_	_	3.1
	ueue Clearance Time (g $_{\rm S}$), s een Extension Time (g $_{\rm E}$), s			_	-	0.0			0.0		-	13.0		_	4.1
		(<i>g</i> e), S		-		0.0	-	-	0.0	-		1.00	-	_	1.00
Phase Call Proba				-			-	-		-			-	_	0.02
Max Out Probab	шц											1.00			0.02
Movement Grou	ıp Res	sults			EB			WE	3		NB			SB	
Approach Mover	nent			L	Т	R	L	Т	R	L	T	R	L	Т	R
Assigned Movem	nent			5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow R	ate (v), veh/h		9	485	100	18	792	791	177	23			28	
Adjusted Saturat	tion Flo	ow Rate (s), veh/h/l	n	341	1809	1675	962	190	0 1892	1491	1665			1425	
Queue Service T	īme (g	g s), s		1.0	3.3	1.3	0.5	15.0	15.1	8.9	1.1			1.0	
Cycle Queue Cle	earance	e Time (<i>g ε</i>), s		16.0	3.3	1.3	3.7	15.0	15.1	11.0	1.1			2.1	
Green Ratio (g/0	C)			0.77	0.77	0.77	0.77	0.77	7 0.77	0.12	0.12			0.12	
Capacity (c), ve	eh/h			285	2773	1284	783	145	7 1451	228	204			247	
Volume-to-Capa				0.032	0.175	_	0.023	0.54		0.776	0.113			0.113	
		t/ln (95 th percentile	,	3.6	36.6	14.9	3.5	192.	_	208.6	20			24.6	
		eh/In (95 th percenti		0.1	1.5	0.6	0.1	7.7		8.3	0.8			1.0	
		RQ) (95 th percent	ile)	0.00	0.00	0.00	0.00	0.00	_	0.00	0.00		-	0.00	
Uniform Delay (7.4	2.8	2.6	3.3	4.2	_	41.3	35.2			35.6	
Incremental Dela		·		0.2	0.1	0.1	0.1	1.5		14.1	0.1		_	0.1	
Initial Queue Del				0.0	0.0	0.0	0.0	0.0		0.0	0.0			0.0	
Control Delay (o				7.6	3.0	2.7	3.4	5.7		55.4	35.2			35.7	
Level of Service				A 2.0	A	A	A	A	A	E 52.	D	D	25.7	D 7	
Approach Delay, Intersection Dela				3.0		A	5.6 .3		Α	53.1		D	35.7 A		D
microection Dela	ay, 5/VE	лі / LOS				9	.u						Λ		
Multimodal Res	ults				EB			WE	3		NB			SB	
Pedestrian LOS	Score	/ LOS		1.83	3	В	1.6	1	В	2.30)	В	2.45	5	В
Bicycle LOS Sco	re / LC)S		0.98	3	Α	1.8		В	0.82	2	Α	0.53	3	Α

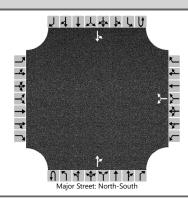
		HCS	Sigr	nalize	d Inte	ersect	ion R	esu	lts Sun	nmary					
General Inform	nation								Intersec	tion Inf	ormatio	nn -	at the state of th	4 가하 t	يا م
Agency	iation	ATS							Duration		0.250			4	
Analyst		RLA		Analys	sic Date	e Nov 3	2022		Area Typ		Other				k.
Jurisdiction		MDT		Time F			ild PM I	Dook	_	, c	1.00			w∳E	<u></u>
Urban Street		HWY 287		_		r 2025	IIIU FIVI	Can	Analysis	Period	1> 7:0	20			, — ←
Intersection		Crossroads Parkwy	,	File Na			287PMn	obuil		i enou	1- 7.0				<u></u>
Project Descrip	tion	MVM		T LIIE IN	aiiie	110012	207 F WIII	Obuli	u.xus				-) <i>†</i> বিকিপ	۳ ا
Demand Inform	nation				EB			W	/B	7	NB			SB	
Approach Move				L	T	R	L		ΓR	L	T	R	L	T	R
Demand (v), v				5	1406		9	68	_	95	5	5	45	0	5
Demand (v), v	CII/II			J	1400	100	9	00	D9 14	93	J J		40	U	3
Signal Informa	ition														<u> </u>
Cycle, s	90.0	Reference Phase	2		Ħ.	T 51	a						$\boldsymbol{\leftrightarrow}$		stz
Offset, s	0	Reference Point	End	Green	73.0	7.0	0.0	0.0	0.0	0.0		1	X Z	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.0	0.0	0.0		0.0			→		松
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0	0.0	0.0		5	6	7	8
Times Desults				EDI	_	CDT	\\/D		\A/DT	ND		NDT	CDI		CDT
Timer Results Assigned Phase				EBI	-	EBT 2	WB	<u> </u>	WBT 6	NBI	_	NBT 8	SBI	-	SBT 4
Case Number	U			_		5.0		\rightarrow	6.0			6.0			8.0
Phase Duration						78.0		-	78.0		_	12.0		_	12.0
Change Period		- \ c		_		5.0	_		5.0	_		5.0		_	5.0
					_	0.0		-	0.0		-	3.1		_	3.1
	x Allow Headway (<i>MAH</i>), s eue Clearance Time (<i>g</i> _s), s					0.0		-	0.0			9.0			5.1
	ueue Clearance Time (g $_{\rm s}$), s reen Extension Time (g $_{\rm e}$), s					0.0		_	0.0		_	0.0	_	_	0.0
Phase Call Pro		(9 =), 3				0.0			0.0			0.98			0.98
Max Out Proba												1.00		_	1.00
Movement Gro		sults			EB			WE	3		NB			SB	
Approach Move				L	Т	R	L	Т	R	L	Т	R	L	T	R
Assigned Move				5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F		,		5	1406		9	353	_	95	10			50	\square
		ow Rate (s), veh/h/l	n	786	1809	1675	405	190		1491	1743			1460	
Queue Service		- ,		0.1	10.8	1.1	0.6	3.9	_	3.9	0.5		_	2.6	
Cycle Queue C		e Time (<i>g c</i>), s		4.0	10.8	1.1	11.4	3.9	_	7.0	0.5		-	3.1	
Green Ratio (g				0.81	0.81	0.81	0.81	0.8		0.08	0.08		_	0.08	
Capacity (c), v				684	2934	1358	360	154		145	136		-	190	
Volume-to-Capa			١	0.007	0.479	_	0.025	0.22		0.654	0.074		-	0.264 47.1	
	· ·	t/ln(95 th percentile eh/ln(95 th percenti		0.8	92.9	9.4	2.5 0.1	38 1.5		108.1 4.3	9.2			1.9	
		RQ) (95 th percent		0.00	0.00	0.00	0.00	0.0		0.00	0.00			0.00	
Uniform Delay (110)	2.4	2.6	1.7	4.4	2.0		43.6	38.5			39.8	
Incremental De	`			0.0	0.6	0.1	0.1	0.3	\rightarrow	8.1	0.1			0.3	
Initial Queue De				0.0	0.0	0.0	0.0	0.0		0.0	0.0			0.0	
Control Delay (·		2.5	3.2	1.8	4.5	2.3	_	51.7	38.6			40.0	
Level of Service				Α	Α	A	Α	Α	А	D	D			D	
Approach Delay				3.1		Α	2.3		Α	50.5		D	40.0		D
Intersection De						5	.7						A		
Multimodal Re	eulte				EB			WE	2		NB			SB	
Pedestrian LOS		/1.08		1.82		В	1.59	_	B	2.30		В	2.46		В
Bicycle LOS Sc				1.02	-	В	1.08	_	A	0.66		A	0.57	_	A
Dicycle LOS 30	OIG / LC	J-0		1.73	,	ט	1.00			0.00	,		0.57		$\overline{}$

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Alpine View
Analysis Year	2025	North/South Street	Alice
Time Analyzed	AM Peak No Build	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



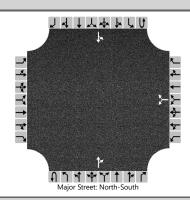
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						1		13			121	0		3	68	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						()									
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up Ho	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)							15							3		
Capacity, c (veh/h)							903							1447		
v/c Ratio							0.02							0.00		
95% Queue Length, Q ₉₅ (veh)							0.1							0.0		
Control Delay (s/veh)							9.1							7.5	0.0	
Level of Service (LOS)							А							А	А	
Approach Delay (s/veh)		9.1												0	.3	
Approach LOS					A							A				

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Alpine View
Analysis Year	2025	North/South Street	Alice
Time Analyzed	PM Peak No Build	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



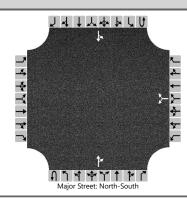
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						1		7			116	1		11	100	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						()									
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)							9							12		
Capacity, c (veh/h)							890							1453		
v/c Ratio							0.01							0.01		
95% Queue Length, Q ₉₅ (veh)							0.0							0.0		
Control Delay (s/veh)							9.1							7.5	0.1	
Level of Service (LOS)							А							А	А	
Approach Delay (s/veh)	9.1													0	.8	
Approach LOS					A				İ				A			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Rankin
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Jeannette Rankin
Analysis Year	2025	North/South Street	Alice
Time Analyzed	AM Peak No Build	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



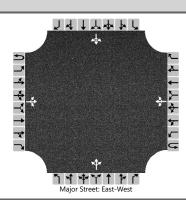
Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						9		37			84	17		39	30	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						()									
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)							50							42		
Capacity, c (veh/h)							903							1474		
v/c Ratio							0.06							0.03		
95% Queue Length, Q ₉₅ (veh)							0.2							0.1		
Control Delay (s/veh)							9.2							7.5	0.2	
Level of Service (LOS)							А							А	А	
Approach Delay (s/veh)					9.2							4.3				
Approach LOS					А							A				

	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	RLA	Intersection	Alice and Rankin							
Agency/Co.	ATS	Jurisdiction	Lewis and Clark							
Date Performed	8/2/2022	East/West Street	Jeannette Rankin							
Analysis Year	2025	North/South Street	Alice							
Time Analyzed	PM Peak No Build	Peak Hour Factor	0.92							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description	Mountain View									



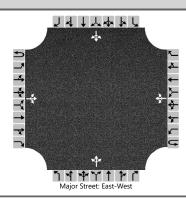
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	oound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						9		61			59	5		41	60	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						(0									
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	T						76							45		
Capacity, c (veh/h)							952							1525		
v/c Ratio							0.08							0.03		
95% Queue Length, Q ₉₅ (veh)							0.3							0.1		
Control Delay (s/veh)							9.1							7.4	0.2	
Level of Service (LOS)							А							А	А	
Approach Delay (s/veh)						9.1							3.2			
Approach LOS					A								A			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Alpine
Time Analyzed	AM Peak No Build	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adjust	stme	nts														
Approach		Eastb	ound			Westk	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		0	96	5		5	18	1		5	0	14		4	0	0
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										()			()	
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up Hea	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)		0				5					21				4	
Capacity, c (veh/h)		1589				1474					910				805	
v/c Ratio		0.00				0.00					0.02				0.01	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.1				0.0	
Control Delay (s/veh)		7.3	0.0	0.0		7.5	0.0	0.0			9.0				9.5	
Level of Service (LOS)		А	Α	А		Α	А	А			А				Α	
Approach Delay (s/veh)	0.0				1.	.6		9.0 9.5			9.5					
Approach LOS	A A							A A				4				

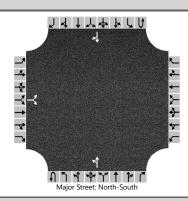
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Alpine
Time Analyzed	PM Peak No Build	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adju	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		0	15	9		14	70	4		4	0	9		0	0	3
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										()				0	
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				15					14				3	
Capacity, c (veh/h)		1511				1582					972				980	
v/c Ratio		0.00				0.01					0.01				0.00	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.0				0.0	
Control Delay (s/veh)		7.4	0.0	0.0		7.3	0.1	0.1			8.8				8.7	
Level of Service (LOS)		А	А	Α		А	Α	А			А				А	
Approach Delay (s/veh)	0.0 1.2							8.8				8.7				
Approach LOS		A A						A A								

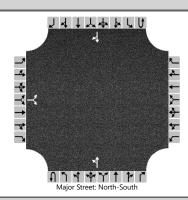
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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Hwy 282
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Hwy 282
Time Analyzed	AM Peak No Build	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume (veh/h)		47		67						5	104				209	14
Percent Heavy Vehicles (%)		3		3						3						
Proportion Time Blocked																
Percent Grade (%)			0													
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		6.43		6.23						4.13						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)			124							5						
Capacity, c (veh/h)			723							1318						
v/c Ratio			0.17							0.00						
95% Queue Length, Q ₉₅ (veh)			0.6							0.0						
Control Delay (s/veh)			11.0							7.7	0.0					
Level of Service (LOS)			В							Α	Α					
Approach Delay (s/veh)		1	1.0							0	.4					
Approach LOS			В							,	4					

	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	RLA	Intersection	Rundle and Hwy 282							
Agency/Co.	ATS	Jurisdiction	Lewis and Clark							
Date Performed	8/2/2022	East/West Street	Rundle							
Analysis Year	2025	North/South Street	Hwy 282							
Time Analyzed	PM Peak No Build	Peak Hour Factor	0.92							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description	Mountain View									

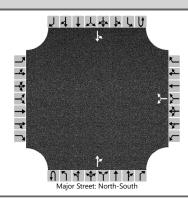


Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume (veh/h)		6		34						72	191				32	15
Percent Heavy Vehicles (%)		3		3						3						
Proportion Time Blocked																
Percent Grade (%)		(0													
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		6.43		6.23						4.13						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)			43							78						
Capacity, c (veh/h)			913							1549						
v/c Ratio			0.05							0.05						
95% Queue Length, Q ₉₅ (veh)			0.1						Ì	0.2						
Control Delay (s/veh)			9.1							7.4	0.4					
Level of Service (LOS)			А							А	А					
Approach Delay (s/veh)	9.1							2.3								
Approach LOS		А							А							

		HCS	S Sigr	nalize	d Inte	ersect	ion R	esul	lts Sur	nmary						
Company Inform	4!								Intonos	diam lud	4! -			4 가수 1	Ja L	
General Inform		ATO							Intersec					4		
Agency		ATS		A I	:- D-4-	N 0	0000		Duration	<u>, </u>	0.250		_3 3.		R_	
Analyst		RLA				Nov 3			Area Typ	oe	Other		→ -* 	w∱E	<u></u>	
Jurisdiction		MDT		Time F	erioa	Dev.	eak Witl	1	PHF		1.00		→	8 8		
Urban Street		HWY 287		Analys	is Year				Analysis	Period	1> 7:0	00	-	ካ ቱ	R	
Intersection		Crossroads Parkwy	1	File Na	ame	HWY2	287AMv	vith.xu	ıs					4147	7 1	
Project Descrip	tion	MVM														
Demand Inform	nation				EB			W	В		NB			SB		
Approach Move	ement			L	Т	R	L	T	R	L	Т	R	L	T	R	
Demand (v), v	eh/h			9	485	130	20	150	65 18	261	5	23	23	0	5	
Signal Informa	ition						T	T		Т					1	
Cycle, s	90.0	Reference Phase	2	1		***	2						4		4	
Offset, s	0	Reference Point	End	Green	60.0	20.0	0.0	0.0	0.0	0.0		1	2	3	4	
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.0	0.0	0.0		0.0	-		→		KÎZ	
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0		0.0		5	6	7	8	
Timer Results				EBI		EBT	WB	L	WBT	NBI	-	NBT	SBI	-	SBT	
Assigned Phase Case Number	e			_	_	2	_	-	6	-		8	_	_	4	
	Phase Duration, s				-	5.0 65.0		-	6.0		-	6.0 25.0		-	8.0 25.0	
	Phange Period, ($Y+R_c$), s					5.0		-	5.0			5.0	_		5.0	
	Change Period,(Y+R ɛ), s Max Allow Headway(MAH), s				_	0.0		_	0.0			3.1		_	3.1	
Queue Clearan		· · · · · · · · · · · · · · · · · · ·				0.0			0.0			19.5			4.0	
Green Extensio		, - ,			$\overline{}$	0.0		_	0.0			0.4		_	0.5	
Phase Call Prol		(3 - 7)										1.00			1.00	
Max Out Proba	bility							1				0.01			0.00	
Movement Gro	up Res	sults			EB			WE	3		NB			SB		
Approach Move				L	Т	R	L	Т	R	L	Т	R	L	Т	R	
Assigned Move	ment			5	2	12	1	6	16	3	8	18	7	4	14	
Adjusted Flow F	Rate (v), veh/h		9	485	130	20	792	791	261	28			28		
Adjusted Satura	ation Flo	ow Rate (s), veh/h/l	n	341	1809	1675	962	1900	1892	1491	1655			1447		
Queue Service	Time (g	g s), s		1.4	4.7	2.5	0.7	21.6	3 21.7	15.3	1.2			0.8		
Cycle Queue C		e Time (<i>g c</i>), s		23.1	4.7	2.5	5.4	21.6		17.5	1.2			2.0		
Green Ratio (g				0.67	0.67	0.67	0.67	0.67		0.22	0.22			0.22	\perp	
Capacity (c), v				225	2403	1112	669	1262	_	378	372			398	\vdash	
Volume-to-Capa			`	0.040	0.202	0.117	0.030	0.62	_	0.690	0.075		_	0.070	\vdash	
	` ,	t/ln (95 th percentile		5.6	69.5	36.6	6.7	316. 12.7		231.4 9.3	0.8		_	21.2 0.8		
<u> </u>	<u> </u>	eh/ln (95 th percenti RQ) (95 th percent		0.2	0.00	0.00	0.3	0.00		0.00	0.00		_	0.00	\vdash	
Uniform Delay (•		uie)	15.4	5.9	5.5	6.9	8.7	_	34.9	27.5		_	27.9	\vdash	
Incremental De	` '			0.3	0.2	0.2	0.3	2.4		0.8	0.0			0.0	\vdash	
	- ,	·		0.0	0.0	0.0	0.0	0.0		0.0	0.0			0.0		
	Initial Queue Delay (d 3), s/veh Control Delay (d), s/veh			15.7	6.0	5.7	7.0	11.1		35.7	27.6			27.9		
Level of Service (LOS)			В	Α	Α	Α	В	В	D	С			С			
Approach Delay, s/veh / LOS			6.1		Α	11.0)	В	35.0)	С	27.9)	С		
Intersection De	lay, s/ve	eh / LOS				12	2.7						В			
Multimodal Re	sulte				EB			WE	}		NB			SB		
Pedestrian LOS		/LOS		1.86		В	1.64	-	, В	2.29		В	2.44		В	
				1.00		A	1.8	_	В	0.96	-	A	0.53	_	A	
2.5,510 200 00	icycle LOS Score / LOS			1.00			1.0			3.50			3.00			

	HCS	S Sigr	nalize	d Inte	ersect	ion R	esul	ts Sun	nmary						
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General Information	ATO						\rightarrow	Intersec				- 1	4	, x	
Agency	ATS		A I	:- D-4-	N 0	0000		Duration	<u> </u>	0.250				R.	
Analyst	RLA				Nov 3			Area Typ	e	Other		→ - ⁷	w∱E	<u>~</u> }-	
Jurisdiction	MDT		Time F	Period	Dev.	eak With	١	PHF		1.00			W † E		
Urban Street	HWY 287		Analys	is Year				Analysis	Period	1> 7:0	00	1	ሻ 🎋	, r	
Intersection	Crossroads Parkwy	/	File Na	ame	HWY2	287PMw	vith.xu	s					4144	7	
Project Description	MVM														
Demand Information				EB			W	В		NB			SB		
Approach Movement			L	Т	R	L	Т	R	L	Т	R	L	Т	R	
Demand (v), veh/h			5	1406	197	14	68	9 14	151	5	7	45	0	5	
Signal Information				T R		T			Г						
Cycle, s 90.0	Reference Phase	2	1	<u> </u>		21						4		4	
Offset, s 0	Reference Point	End		72.0	7.0						1	2	3	4	
Uncoordinated No	Simult. Gap E/W	On	Green Yellow		7.0	0.0	0.0		0.0			→		r ∱a	
Force Mode Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0		0.0		5	6	7	8	
			II.												
Timer Results			EBI		EBT	WB	L	WBT	NBI	_	NBT	SBI	_	SBT	
Assigned Phase				_	2		_	6			8			4	
Case Number				_	5.0	_	+	6.0	_	_	6.0		+	8.0	
Phase Duration, s			_	_	78.0	_	-	78.0	_		12.0	_	+	12.0	
Change Period, (Y+R c), s			-	-	5.0	-	+	5.0	-	-	5.0		+	5.0	
Max Allow Headway (·			_	0.0		-	0.0	-		3.1		_	3.1	
Queue Clearance Time Green Extension Time	, = ,		-	-	0.0	-	+	0.0	-	-	9.0		+	5.2 0.1	
Phase Call Probability	,				0.0		+	0.0			1.00			1.00	
Max Out Probability				_			_			_	1.00		_	1.00	
Movement Group Res	sults			EB			WB	-		NB			SB	$\overline{}$	
Approach Movement			L	T	R	L	Т	R	L	T	R	L	Т	R	
Assigned Movement	\		5	2	12	1	6	16	3	8	18	7	4	14	
Adjusted Flow Rate (v		l	5	1406	197	14	353		151	12		_	50	\vdash	
Adjusted Saturation Flo Queue Service Time (ın	786	1809	1675	405 1.0	1900 3.9	3.9	1491 3.8	1719 0.6			1435 2.6	\vdash	
Cycle Queue Clearance			0.1 4.0	10.8	2.3	11.8	3.9	3.9	7.0	0.6			3.2	\vdash	
Green Ratio (g/C)	λο τιπιο (g ε), s		0.81	0.81	0.81	0.81	0.81		0.08	0.08		-	0.08	+	
Capacity (c), veh/h			684	2934	1358	360	1541		143	134			188	\vdash	
Volume-to-Capacity Ra	atio (X)		0.007	0.479	0.145	0.039	0.22	_	1.053	0.090			0.266	$\overline{}$	
Back of Queue (Q), f	· ,)	0.8	92.9	19.8	3.9	38	37.8	279.1	11			47.2		
Back of Queue (Q), v	· · · · · · · · · · · · · · · · · · ·	•	0.0	3.7	0.8	0.2	1.5	1.5	11.2	0.4			1.9		
Queue Storage Ratio (RQ)(95 th percen	tile)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			0.00		
Uniform Delay (d 1), s	s/veh		2.4	2.6	1.8	4.4	2.0	2.0	44.2	38.5			39.8		
Incremental Delay (d :	<u> </u>		0.0	0.6	0.2	0.2	0.3	0.3	89.9	0.1			0.3		
Initial Queue Delay (d	<u>, </u>		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			0.0	igspace	
Control Delay (d), s/veh			2.5 A	3.2	2.0	4.7	2.3	2.3	134.1	38.6			40.1		
	Level of Service (LOS)			A	A	A 0.4	A	A	F	D	_	40	D		
Approach Delay, s/veh			3.0		A 1	2.4		A	127.	U	F	40.1		D	
Intersection Delay, s/ve	en / LOS				17	1.5						В			
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS		1.82	2	В	1.59		В	2.30		В	2.46	5	В		
redesiliali LOS Scole	cycle LOS Score / LOS														

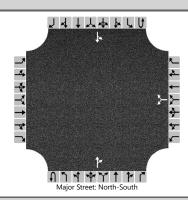
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Alpine View
Analysis Year	2025	North/South Street	Alice
Time Analyzed	AM Peak With Dev.	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastk	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						4		68			154	1		23	80	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						(0									
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa															
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	T						78							25		
Capacity, c (veh/h)							859							1403		
v/c Ratio							0.09							0.02		
95% Queue Length, Q ₉₅ (veh)							0.3							0.1		
Control Delay (s/veh)							9.6							7.6	0.1	
Level of Service (LOS)							А							А	А	
Approach Delay (s/veh)						9	.6							1	.8	
Approach LOS	9.6 A												,	Α		

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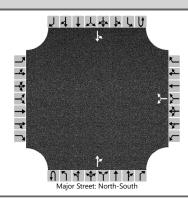
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Alpine View
Analysis Year	2025	North/South Street	Alice
Time Analyzed	PM Peak With Dev.	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						3		45			137	5		76	137	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						()									
Right Turn Channelized																
Median Type Storage	Undivided															
Critical and Follow-up He	adwa															
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)							52							83		
Capacity, c (veh/h)							854							1420		
v/c Ratio							0.06							0.06		
95% Queue Length, Q ₉₅ (veh)							0.2							0.2		
Control Delay (s/veh)							9.5							7.7	0.5	
Level of Service (LOS)							Α							А	А	
Approach Delay (s/veh)		9.5										.1				
Approach LOS	9.5 A												,	4		

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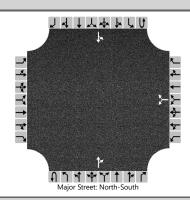
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Rankin
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Jeannette Rankin
Analysis Year	2025	North/South Street	Alice
Time Analyzed	AM peak projected	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						12		70			85	18		50	33	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						()									
Right Turn Channelized																
Median Type Storage		Undivided														
Critical and Follow-up H	eadwa	dways														
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)							89							54		
Capacity, c (veh/h)							906							1471		
v/c Ratio							0.10							0.04		
95% Queue Length, Q ₉₅ (veh)							0.3							0.1		
Control Delay (s/veh)							9.4							7.5	0.3	
Level of Service (LOS)							А							А	Α	
Approach Delay (s/veh)	9.4											4.7				
Approach LOS		3.4 A							A							

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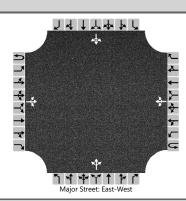
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Alice and Rankin
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Jeannette Rankin
Analysis Year	2025	North/South Street	Alice
Time Analyzed	PM peak projected	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LR					TR		LT		
Volume (veh/h)						11		82			59	9		79	62	
Percent Heavy Vehicles (%)						3		3						3		
Proportion Time Blocked																
Percent Grade (%)						()									
Right Turn Channelized																
Median Type Storage		Undivided														
Critical and Follow-up He	eadwa															
Base Critical Headway (sec)						7.1		6.2						4.1		
Critical Headway (sec)						6.43		6.23						4.13		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.53		3.33						2.23		
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)							101							86		
Capacity, c (veh/h)							931							1519		
v/c Ratio							0.11							0.06		
95% Queue Length, Q ₉₅ (veh)							0.4							0.2		
Control Delay (s/veh)							9.3							7.5	0.4	
Level of Service (LOS)							Α							А	Α	
Approach Delay (s/veh)					9.3							4.4				
Approach LOS						,	4		i .				А			

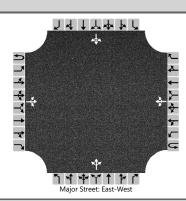
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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Alpine
Time Analyzed	AM peak projected	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		0	103	5		5	21	10		5	0	14		28	0	0
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up Ho	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				5					21				30	
Capacity, c (veh/h)		1571				1465					898				786	
v/c Ratio		0.00				0.00					0.02				0.04	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.1				0.1	
Control Delay (s/veh)		7.3	0.0	0.0		7.5	0.0	0.0			9.1				9.8	
Level of Service (LOS)		А	А	А		А	А	А			А				А	
Approach Delay (s/veh)		0	.0			1	.1			9	.1			9	.8	
Approach LOS		,	4			,	4			,	4				Ą	

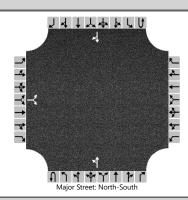
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Alpine View
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Alpine
Time Analyzed	PM peak projected	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		0	19	9		14	77	32		5	0	9		0	0	18
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized																
Median Type Storage		Undivided														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				15					15				20	
Capacity, c (veh/h)		1463				1576					928				951	
v/c Ratio		0.00				0.01					0.02				0.02	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.0				0.1	
Control Delay (s/veh)		7.5	0.0	0.0		7.3	0.1	0.1			8.9				8.9	
Level of Service (LOS)		А	А	А		А	А	А			А				А	
Approach Delay (s/veh)		0	.0			0	.9			8	.9		8.9			
Approach LOS		A A							A A							

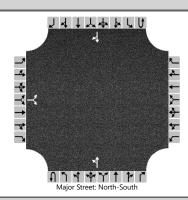
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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Hwy 282
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Hwy 282
Time Analyzed	AM peak projected	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0	
Configuration			LR							LT						TR	
Volume (veh/h)		58		86						12	104				209	18	
Percent Heavy Vehicles (%)		3		3						3							
Proportion Time Blocked																	
Percent Grade (%)		(0														
Right Turn Channelized																	
Median Type Storage		Undivided															
Critical and Follow-up He	eadwa	ys															
Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		6.43		6.23						4.13							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.53		3.33						2.23							
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)			157							13							
Capacity, c (veh/h)			714							1313							
v/c Ratio			0.22							0.01							
95% Queue Length, Q ₉₅ (veh)			0.8							0.0							
Control Delay (s/veh)			11.4							7.8	0.1						
Level of Service (LOS)			В							А	Α						
Approach Delay (s/veh)		11.4								0.9							
Approach LOS		В						A									

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Hwy 282
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Hwy 282
Time Analyzed	PM peak projected	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		

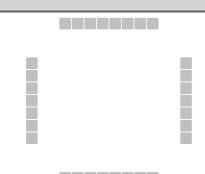


Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume (veh/h)		13		47						95	191				32	28
Percent Heavy Vehicles (%)		3		3						3						
Proportion Time Blocked																
Percent Grade (%)			0													
Right Turn Channelized																
Median Type Storage		Undivided														
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		6.43		6.23						4.13						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.53		3.33						2.23						
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	Π		65							103						
Capacity, c (veh/h)			837							1530						
v/c Ratio			0.08							0.07						
95% Queue Length, Q ₉₅ (veh)			0.3		Ì				Ì	0.2						
Control Delay (s/veh)			9.7							7.5	0.6					
Level of Service (LOS)			А		Ì				Ì	А	А					
Approach Delay (s/veh)	9.7								2	.9						
Approach LOS		Α								A						

	General Information Agency ATS			d Inte	rsect	ion R	esul	ts Sun	nmary					
O									41 a a las£				4 사하 t	la I.
							\rightarrow	Intersec		1			4	4. 7
			Δ		N. O	0000		Duration	<u> </u>	0.250				<u> </u>
Analyst	RLA				Nov 3		\rightarrow	Area Typ	e e	Other			w∱E	<u>~</u> _}
Jurisdiction	MDT		Time F		Dev.	eak With		PHF		1.00			-W†= 8	
Urban Street	HWY 287		Analys	sis Year	DOUE	3LE 202	25	Analysis	Period	1> 7:0	00	N.	5 ቱ	, s
Intersection	Crossroads Parkwy	/	File N	ame	HWY2	287AMw	vith2x.	xus					11147	7
Project Description	MVM													
Demand Information	n			EB			WI	В		NB			SB	
Approach Movement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), veh/h			9	485	260	40	156	55 18	520	10	50	23	0	5
Signal Information				T R		T	<u> </u>	<u> </u>	<u> </u>					
Cycle, s 90.0	Reference Phase	2	1	14 £		21						4		4
Offset, s 0	Reference Point	End	C	40.0	24.0						1	2	3	4
Uncoordinated No	Simult. Gap E/W	On	Green Yellow		31.0 4.0	0.0	0.0		0.0	-		→		c ∱a
Force Mode Fixe	d Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0		0.0		5	6	7	8
Timer Results			EBI		EBT	WB	L	WBT	NBI	L	NBT	SBI		SBT
Assigned Phase					2	\perp	_	6			8			4
Case Number				_	5.0	_	_	6.0		_	6.0			8.0
Phase Duration, s			_	_	54.0	_	+	54.0		+	36.0		_	36.0
Change Period, (Y+Rc), s				-	5.0	_	+	5.0		-	5.0			5.0
Max Allow Headway	<u> </u>		_	_	0.0	_	+	0.0	_	_	3.1	_		3.1
Queue Clearance Tir	, - ,			-	0.0		-	0.0		-	33.0			1.2
Green Extension Tim Phase Call Probabilit	, = ,				0.0	_	+	0.0			1.00			1.00
Max Out Probability	У			_		-	+			_	1.00		_	0.00
Wax out Fobability											1.00			0.00
Movement Group R				EB			WB			NB			SB	
Approach Movement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Movement			5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (9	485	260	40	792		520	60		_	28	
	Flow Rate (s), veh/h/	In	341	1809	1675	962	1900		1491	1652			1381	
Queue Service Time Cycle Queue Clearar	· - /		1.9 31.3	6.3	7.5 7.5	2.1 8.4	29.3 29.3	_	28.2	2.2		_	0.6 2.8	
Green Ratio (g/C)	ice fille (g c), s		0.54	0.54	0.54	0.54	0.54	_	0.34	0.34			0.34	
Capacity (c), veh/h			154	1970	912	536	1034		546	569			548	
Volume-to-Capacity F	Ratio (X)		0.058	_	0.285	0.075	0.766	_	0.952	0.105			0.051	
	ft/ln (95 th percentile	e)	8.2	107.9	123.4	20.7	466.9		542	37.4			17.7	
	veh/ln (95 th percent	•	0.3	4.3	4.9	0.8	18.7		21.7	1.5			0.7	
	(RQ)(95 th percen		0.00	0.00	0.00	0.00	0.00		0.00	0.00			0.00	
Uniform Delay (d 1),	s/veh		28.3	10.8	11.1	13.0	16.0	16.0	32.4	20.1			20.3	
Incremental Delay (d 2), s/veh		0.7	0.3	0.8	0.3	5.4	5.5	26.6	0.0			0.0	
Initial Queue Delay (· · · · · · · · · · · · · · · · · · ·		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			0.0	
	Control Delay (d), s/veh			11.1	11.8	13.3	21.4		58.9	20.1			20.3	
	Level of Service (LOS)			В	В	В	С	C	E	С			С	
	Approach Delay, s/veh / LOS			6	В	21.3	3	С	54.9	9	D	20.3	3	С
Intersection Delay, s/	Intersection Delay, s/veh / LOS				25	5.3						С		
Multimodal Results	ultimodal Results			EB			WB			NB			SB	
Pedestrian LOS Scor	re / LOS		1.89	9	В	1.66	3	В	2.28	3	В	2.43	3	В
Bicycle LOS Score /	LOS		1.11		Α	1.83	3	В	1.44	1	Α	0.53	3	Α

		HCS	S Sigr	nalize	d Inte	rsect	ion R	esu	lts Sur	nmary	,				
General Inforn	nation								Interse	tion Inf	ormatio	on		1 1 * 1 * 4 * 1	له لي
Agency		ATS							Duration	n, h	0.250			*	
Analyst		RLA		Analys	sis Date	Nov 3	, 2022		Area Ty	oe .	Other		,		A. 10
Jurisdiction		MDT		Time F			eak With	1	PHF		1.00		→ ** → → → →	w } E	←
Urban Street		HWY 287		Analys	sis Year	DOUE	BLE 202	5	Analysis	Period	1> 7:0	00		ካ ቱ	Į.
Intersection		Crossroads Parkwy	'	File Na	ame	HWY2	287PMw	/ith2x	.xus					14147	7
Project Descrip	tion	MVM													
Demand Inforr	nation				EB		T	W	'B		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), v				5	1406		30	68	_	300	10	15	45	0	5
Bornaria (+), +	011/11				1100	100			, ,	000	10	10	10		
Signal Informa	ation				T _ 5		T			\top					1
Cycle, s	90.0	Reference Phase	2		₩.	54	71						4		ςŢХ
Offset, s	0	Reference Point	End	Green	59.0	21.0	0.0	0.0	0.0	0.0		1	¥ 2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.0	0.0	0.0		0.0			}		кtя
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0		0.0		5	6	7	8
Timer Beaute				EDI		EDT	VA/D		WDT	NIDI		NIDT	CD		CDT
Timer Results	0			EBI	-	EBT 2	WB	_	WBT 6	NBI	-	NBT 8	SBI	-	SBT 4
Assigned Phase Case Number				-			-	-		-			-		
			_	-	5.0	_	+	6.0	-	-	6.0	_		8.0	
	Phase Duration, s			_	_	64.0	_	+	64.0	-	_	26.0	_	_	26.0
Change Period		,				5.0			5.0			5.0			5.0
Max Allow Head Queue Clearan						0.0				0.0		3.1			3.1 5.2
Green Extension		, - ,		_	_	0.0	_	-	0.0	_		0.0	-		0.6
Phase Call Pro		(g e), s		-		0.0	-	-	0.0	-		1.00			1.00
Max Out Proba					_			_			_	1.00			0.00
Movement Gro	oup Res	ults			EB			WE	3		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Assigned Move	ment			5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow I	Rate (v), veh/h		5	1406	400	30	353	350	300	25			50	
Adjusted Satura	ation Flo	ow Rate (s), veh/h/l	n	786	1809	1675	405	190	0 1886	1491	1715			1431	
Queue Service	Time (g	g s), s		0.2	19.7	9.7	4.1	7.1	7.1	17.8	1.0			2.1	
Cycle Queue C		e Time (<i>g c</i>), s		7.3	19.7	9.7	23.8	7.1	_	21.0	1.0			3.2	
Green Ratio (g				0.66	0.66	0.66	0.66	0.66		0.23	0.23			0.23	
Capacity (c), v				534	2372	1098	257	124	_	375	400			410	
Volume-to-Cap		· · · ·		0.009		0.364	0.117	0.28		_	0.062			0.122	
	. ,	/In (95 th percentile		1.9	272.6	142.9	19	116	_	_	18.4			38.1	
	<u> </u>	eh/ln (95 th percenti		0.1	10.9	5.7	0.8	4.6	_	12.0	0.7			1.5	
		RQ) (95 th percent	tile)	0.00	0.00	0.00	0.00	0.00		0.00	0.00		_	0.00	
Uniform Delay	, ,			8.1	8.7	7.0	15.4	6.6		36.2	26.8			27.9	
Incremental De	- 1	<u>, </u>		0.0	1.1	0.9	0.9	0.6	_	10.7	0.0			0.0	
	nitial Queue Delay (d 3), s/veh			0.0	0.0	0.0	0.0	0.0		0.0	0.0		_	0.0	-
Control Delay (d), s/veh				8.1	9.8	8.0	16.4	7.1	_	47.0	26.9			27.9	
Level of Service (LOS)			Α 0.4	A	A	B 7.5	A	A	D 45 (С		07.4	C		
Approach Delay, s/veh / LOS			9.4		A 13	7.5 3.3		A	45.4		D	27.9 B	1	С	
microcolon De	ntersection Delay, s/veh / LOS					13	 						<u>.</u>		
Multimodal Re	sults				EB			WE	3		NB			SB	
Pedestrian LOS	Score	/LOS		1.86	6	В	1.64	-	В	2.29)	В	2.44	4	В
Bicycle LOS Sc	edestrian LOS Score / LOS cycle LOS Score / LOS			1.98	3	В	1.09)	Α	1.02	2	Α	0.57	7	Α

	HCS Two-Way Stop	-Control Report					
General Information		Site Information					
Analyst	RLA	Intersection	Rundle and Hwy 282				
Agency/Co.	ATS	Jurisdiction	Lewis and Clark				
Date Performed	8/2/2022	East/West Street	Rundle				
Analysis Year	2025	North/South Street	Hwy 282				
Time Analyzed	AM DOUBLE projected	Peak Hour Factor	0.92				
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25				
Project Description	Mountain View						



Major Street: North-South

Approach Movement Priority Number of Lanes Configuration Volume (veh/h)	U	L 10 0 120	T 11 1 LR	R 12 0	U	Westl L 7	oound T	R	U	North				South	bound			
Priority Number of Lanes Configuration	U	10	11	12	U		Т	R	11									
Number of Lanes Configuration		0	1			7	T .			L	Т	R	U	L	Т	R		
Configuration			' '	0			8	9	1U	1	2	3	4U	4	5	6		
		120	LR			0	0	0	0	0	1	0	0	0	1	0		
Volume (veh/h)		120								LT						TR		
				170						24	104				209	40		
Percent Heavy Vehicles (%)		3		3						3								
Proportion Time Blocked																		
Percent Grade (%)		(0															
Right Turn Channelized																		
Median Type Storage				Undi	vided													
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)		7.1		6.2						4.1								
Critical Headway (sec)		6.43		6.23						4.13								
Base Follow-Up Headway (sec)		3.5		3.3						2.2								
Follow-Up Headway (sec)		3.53		3.33						2.23								
Delay, Queue Length, an	d Leve	l of Se	ervice															
Flow Rate, v (veh/h)			315							26								
Capacity, c (veh/h)			686							1287								
v/c Ratio			0.46							0.02								
95% Queue Length, Q ₉₅ (veh)			2.4							0.1								
Control Delay (s/veh)			14.6							7.9	0.2							
Level of Service (LOS)			В							А	А							
Approach Delay (s/veh)		14	4.6							1.6								

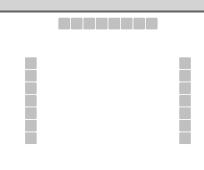
В

Approach LOS

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	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	RLA	Intersection	Rundle and Hwy 282
Agency/Co.	ATS	Jurisdiction	Lewis and Clark
Date Performed	8/2/2022	East/West Street	Rundle
Analysis Year	2025	North/South Street	Hwy 282
Time Analyzed	PM DOUBLE projected	Peak Hour Factor	0.92
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Mountain View		



Major Street: North-South

Vehicle Volumes and Adj	1				Т	344 .1			Т				ı					
Approach		Eastk	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0		
Configuration			LR							LT						TR		
Volume (veh/h)		26		100						200	191				32	60		
Percent Heavy Vehicles (%)		3		3						3								
Proportion Time Blocked																		
Percent Grade (%)			0															
Right Turn Channelized																		
Median Type Storage				Undi	vided													
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)		7.1		6.2						4.1								
Critical Headway (sec)		6.43		6.23						4.13								
Base Follow-Up Headway (sec)		3.5		3.3						2.2								
Follow-Up Headway (sec)		3.53		3.33						2.23								
Delay, Queue Length, an	d Leve	l of S	ervice															
Flow Rate, v (veh/h)			137							217								
Capacity, c (veh/h)			705							1486								
v/c Ratio			0.19							0.15								
95% Queue Length, Q ₉₅ (veh)			0.7							0.5								
Control Delay (s/veh)			11.3							7.8	1.3							
Level of Service (LOS)			В							А	А							
Approach Delay (s/veh)		11.3									4.6							

В

Approach LOS

Α

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